

HYDROLOGY STUDY

For

REIDO FARMS, LLC APN 0521-051-08 Tentative Parcel Map 20538 Daggett, CA

June 2, 2023

Prepared by:

Merrell-Johnson Companies

22221 US Highway 18 Apple Valley, CA 92307 (760) 240-8000

Job No. 3852.001



E. Cary Packer, PE Associate Engineer R.C.E. 51752 Exp. 06/30/24 Mark D. Rowan Project Manager

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SECTION 1

DISCUSSION

INTRODUCTION

The purpose of this study was to determine the impact, if any, of the 100-year storm runoff flow tributary to the project site as delineated on the map contained in this study. The project parcel encompasses approximately 92 acres of property located on the west side of Minneola Road and north of the railroad tracks and Larch Street in the unincorporated area of Daggett, in San Bernardino County, California. The proposed development is to divide the existing parcel into two parcels 0f 67.4 and 24.5 acres.

METHODOLOGY

The method in determining these peak runoff flows was the rational method for areas north of Interstate 40 and the unit hydrograph method for areas south of Interstate 40 as specified in the 1986 San Bernardino County Hydrology Manual and the 2010 San Bernardino County Hydrology Manual Addendum for Arid Regions. The existing offsite flow was examined and delineated from U.S.G.S. Map: Minneola and an examination of the project site.

The tributary watershed areas examined lie southerly and southwesterly of the project site. The primary storm runoff is generated within the tributary areas lying south and southwest of the property between the existing railroad improvements and Interstate 40. The tributary area north of Interstate 40 consists of three primary flowlines, encompassing approximately 652.5 acres. The area south of Interstate 40 encompasses approximately 8,260 acres and multiple blueline streams. Storm runoff from this 8,260 acre area is captured by earthen berms upstream of Interstate 40 which divert the runoff to a central bridge structure on the interstate that allows storm runoff to flow through the freeway right-of-way.

Point rainfalls for the 100-year storm were obtained from the NOAA Atlas 14 per the 2010 Addendum to the County Hydrology Manual. The 100-year 1-hour point rainfall for the site is 1.41". Per the aforementioned addendum, AMC I was used for the project site. Soils types ranged from primarily Type A soils north of Interstate 40, to some areas of Type C & D soils and unmapped soil types on the south side of Interstate 40. Due to the large area of unmapped soil types south of Interstate 40, a conservative value of Soil Type C was used in the unit hydrograph analysis. Rainfall and soil maps are included as exhibits in Section 3 of this report.

The offsite tributary area examined in this study is shown in Table A.

Table A

Sub-area	Elevation Difference (ft.)	Length (ft)	Area (Ac)	Avg. Slope (ft/ft)
Unit Hydrograph – South I40	3,152	46,042	8,260	0.0685
Node 11 – Node 16	140	7,553	162.8	0.0185
Node 21 – Node 26	140	7,039	197.5	0.0199
Node 31 – Node 36	75	5,651	292.2	0.0133

EXISTING CONDITIONS

The project parcel encompasses approximately 92 acres of property located on the west side of Minneola Road and north side of the existing railroad tracks and Larch Street in the unincorporated area of Daggett, in San Bernardino County, California. The property is currently vacant land zoned for rural living. Minneola Road is a paved roadway with dirt shoulders and Larch Street is a dedicated, ungraded road. The existing railroad improvements serve to capture the tributary runoff and convey it easterly to three storm culvert locations beneath the railroad tracks and onto the right-of-way of Larch Street.

The three culvert locations are shown on the tributary watershed map as Node 15, Node 26, and Node 36 located from west to east along the tracks. Node 15 is a 48" RCP and Nodes 26 & 36 consist of three 27" HDPE pipes at each location. The approximate flow capacity of the systems at nodes 15, 26, and 36 is 176 cfs, 156 cfs, and 156 cfs respectively.

Runoff flow through the culvert at Node 15 flows to the western property line and enters the project site as sheet flow along the western boundary of the site. There is no evidence of scour from large, concentrated flows crossing the project.

Runoff flow through culverts at Nodes 26 and 36 are conveyed easterly along the northern side of the tracks following the right-of-way of Larch Street. There is evidence of runoff flows following a natural channel depression between the tracks and Larch Street to Minneola Road. Flows pass under Minneola Road through a 24" CMP storm drain with excess flows crossing over Minneola Road as sheet flow following their historical flow paths.

Storm runoff from the tributary area south of Interstate 40 is concentrated and flows beneath the freeway improvements under a bridge structure. After passing underneath Interstate 40 runoff flows spread to flood plain sheet flows and flow northeasterly toward National Trails Highway and Minneola Road. The runoff flows along National Trails Hwy and the railroad improvements eastward past the project site.

The results of the offsite flow analysis are summarized in Table B.

Table B

Sub-Area	Q ₁₀₀ (cfs)
Unit Hydrograph – South of I-40	4,652
Node 11 – Node 16	125.3
Node 21 – Node 26	183.5
Node 31 – Node 36	89.3

CONCLUSIONS AND RECOMMENDATIONS

During our field investigation of the site, we observed the existing conditions as stated previously. Future development of the project is being performed in conjunction with engineered improvement plans. Off-site flows from the west enter the project site along the western property boundary as sheet flow and flow across the project site then cross Minneola Road towards the Mojave River. Runoff flows passing underneath the railroad tracks through the existing culverts will flow along the north side of the tracks and follow the right-of-way of Larch Street.

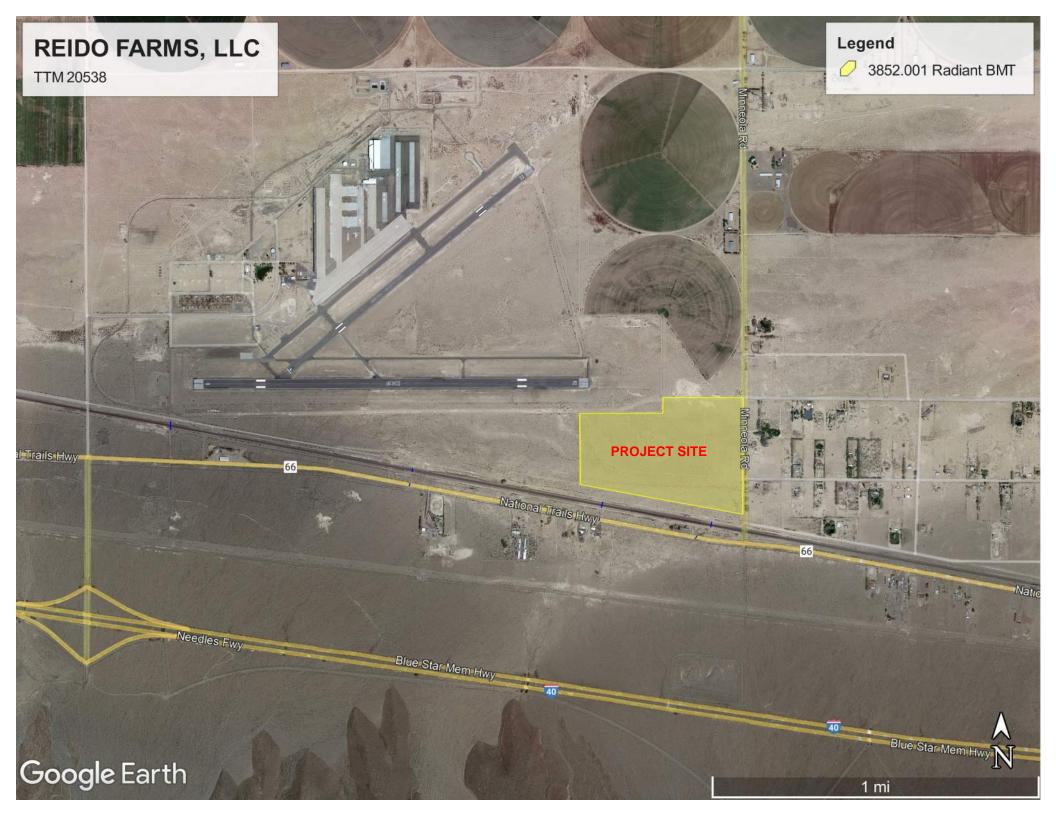
Storm runoff from areas south of Interstate 40 are concentrated as they flow through the freeway improvements and then spread back into a flood plain and sheet flows as the runoff flows northeasterly. Flows will be diverted by the existing railroad improvements and be directed easterly along National Trails Hwy and the railroad tracks.

Future construction within the project should be elevated above the adjacent terrain to protect them from storm runoff in the form of sheet flows. Off-site improvements of Larch Street and Minneola Road will serve to convey the runoff with in historical flow patterns and past the project.

SECTION 2

EXHIBITS

VICINITY MAP



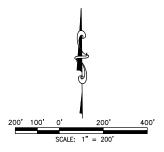
PROPOSED DEVELOPMENT PLAN

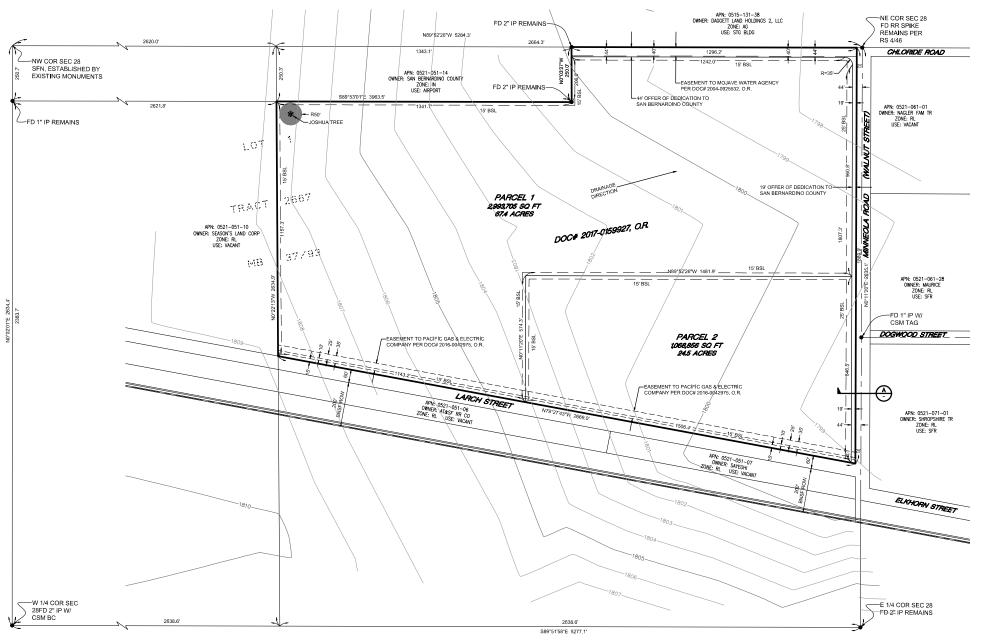
TENTATIVE PARCEL MAP NO. 20538

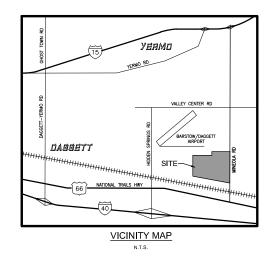
BEING A SUBDIVISION OF LOT 1, TRACT 2667 AS RECORDED IN BOOK 37, PAGE 93, RECORDS OF SAN BERNARDINO COUNTY, STATE OF CALIFORNIA

MERRELL JOHNSON COMPANIES

MAY 2022







LEGEND

_____BSL_____ INDICATES BUILDING SETBACK LINE ---- INDICATES EASEMENTS AS SHOWN INDICATES PROPERTY LINE

PROJECT SCOPE:

- THERE ARE 2 PARCELS TO THIS PARCEL MAP.
 PARCEL 1: VACANT
 PARCEL 2: SOLAR
- 4. THERE ARE NO NEW STREETS ON THIS PARCEL MAP.
- THE AVERAGE SLOPE OF ALL FEASIBLE ACCESS ROUTES AND BUILDING SITES DOES NOT EXCEED (10%.)
- 6. THERE ARE NO WATER COURSES TRAVERSING THE PROPERTY
- THERE ARE NO PROTECTED OR ENDANGERED TREES EXISTING ON THIS SITE.

UTILITIES:

ELECTRIC: EDISON INTERNATIONAL 30553 RIMROCK ROAD BARSTOW, CALIFORNIA 92311 (760) 252-6451

SOUTHWEST GAS CORP. 751 E. MAIN STREET BARSTOW, CALIFORNIA 92311 (760) 256-3571

WATER:

TELEPHONE: FRONTJER COMMUNICATIONS (877) 236-2894

APN 0521-051-08 OWNER/APPLICANT: REIDO FARMS, LLC C/O BRIAN VAIL 2410 FAIR OAKS BLVD., SUITE 110 SACRAMENTO, CA 95825 (916) 379-0955

PREPARED BY: MERRELL JOHNSON COMPANIES 22221 HIGHWAY 18 APPLE VALLEY, CA 92307 (760) 240-8000

MAP PREPARATION DATE: APRIL 26, 2023

EXISTING AC PAVING TYPICAL SECTION A MINEOLA ROAD N.T.S.

LEGAL DESCRIPTION:

ALL OF LOT 1 OF TRACT NO, 2667, EXCEPTING THE WEST 2620 FEET AS MEASURED ALONG THE NORTH LINE OF SAID LOT, IN THE COUNTY OF SAIN BERNARDING, STATE OF CALFORNIA, AS PER MAP RECORDED IN BOOK 37, PAGE 93 OF MAPS, IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY.

ALSO EXCEPTING THAT PORTION THEREOF CONDEMNED BY THE COUNTY OF SAN BERNARDINO FOR AN AIRPORT, DESCRIBED AS FOLLOWS:

THE NORTH 250 FEET OF THE NORTHWEST QUARTER AND THE NORTH 250 FEET OF THE NORTHWEST QUARTER OF THE NORTHEAST QUARTER OF SECTION 28, TOWNSHIP 9 NORTH, RANGE 2 EAST, SAN BERNADING DASE AND MERIDIAN, ACCORDING TO THE GOVERNMENT TOWNSHIP PLAT THEREOF.

ALSO EXCEPTING THEREFROM AN UNDIVIDED 1/2 INTEREST IN OIL, GAS AND ALL MINERAL RIGHTS, AS RESERVED IN A DEED FROM FEDERICK P. HAUSER AND RODMAN WILDE HAUSER TO CECIL C. COOPER AND BESSIE B. COOPER RECORDED JUNE 15, 1988, IN BOOK 2249, PAGE 340, OF OFFICIAL RECORDS:

BENCHMARK

AS MILES WEST ALONG THE ATCHISON, TOPEKA AND SANTA FE RAILWAY FROM THE CROSSING OF MT. VIEW ROAD AT NEWBERRY, 3 FEET WEST SOUTHWEST OF THE 21ST POLE EAST OF MILEPOST 731, 0.5 MILE PAST 70 FTHE CROSSING OF MINNEOLA ROAD, 38 FEET SOUTH OF THE SOUTH RAIL OF THE SOUTH TRACK, 88 FEET NORTH OF THE CENTER LINE OF PAITONAL TRAILS HIGHWAY, 2.0 FEET NORTH OF A WITHESS POST, AGDUT 3 FEET LOWER THAN THE TRACK, AND SET IN THE TOP OF A CONCRETE POST PROJECTING 0.6 FOOT ABOVE THE GROUND. SEC 27, T9N, R2E ELEV=1903.08

BASIS OF BEARINGS

TAKEN FROM THE EAST LINE OF THE NE 1/4 OF SECTION 28, T9N, R2E, SBM, AS SHOWN ON TRACT 2667, MB 37/93
BEING: N0*11'20'E



MERRELL

TENTATIVE PARCEL MAP NO. 20538

LOT 1 TRACT 2667, MB 37/93 APN 0521-051-08 REIDO FARMS, LLC

DRAWN B MM DATE: 04/26/2 JOB NO. 3852.001 SHEET OF 1

SECTION 3

HYDROLOGY CALCULATIONS

OFF-SITE HYDROLOGY CALCULATIONS



RATIONAL CALCULATIONS - Q₁₀₀

San Bernardino County Rational Hydrology Program

(Hydrology Manual Date - August 1986)

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CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989-2004 Version 7.0
    Rational Hydrology Study Date: 06/05/23
______
REIDO FARMS, LLC - JOB 3852.001
OFF-SITE TRIBUTARY RUNOFF FLOW
NODE 11 - NODE 16
100-YEAR STORM EVENT - AMC I
_____
MERRELL JOHNSON COMPANIES
22221 HIGHWAY 18
APPLE VALLEY, CA 92307
(760) 240-8000 * FAX (760) 240-1400
______
******* Hydrology Study Control Information ********
Rational hydrology study storm event year is 100.0
Computed rainfall intensity:
Storm year = 100.00 1 hour rainfall = 1.410 (In.)
Slope used for rainfall intensity curve b = 0.7000
Soil antecedent moisture condition (AMC) = 1
Process from Point/Station 11.000 to Point/Station
**** INITIAL AREA EVALUATION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Initial subarea data:
Initial area flow distance = 986.000(Ft.)
Top (of initial area) elevation = 2049.000(Ft.)
Bottom (of initial area) elevation = 2007.000(Ft.)
Difference in elevation = 42.000(Ft.)
Slope = 0.04260 \text{ s(%)} =
                      4.26
TC = k(0.525)*[(length^3)/(elevation change)]^0.2
Initial area time of concentration = 15.554 min.
Rainfall intensity = 3.628(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area (Q=KCIA) is C = 0.692
Subarea runoff = 13.047(CFS)
Total initial stream area =
                           5.200(Ac.)
Pervious area fraction = 1.000
Initial area Fm value = 0.840(In/Hr)
Process from Point/Station 12.000 to Point/Station
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 0.606(Ft.), Average velocity = 3.556(Ft/s)
```

****** Irregular Channel Data *******

```
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
     1
                0.00
                                1 00
     2.
                10.00
                                0.00
     3
                20.00
                                1.00
Manning's 'N' friction factor = 0.035
______
Sub-Channel flow = 13.047(CFS)
 flow top width = 12.115(Ft.)
velocity= 3.556(Ft/s)
area = 3.669(Sq.Ft)
      ' Froude number = 1.139
Upstream point elevation = 2007.000(Ft.)
Downstream point elevation = 1976.000(Ft.)
Flow length = 893.000(Ft.)
Travel time = 4.19 min.
Time of concentration = 19.74 min.
Depth of flow = 0.606(Ft.)
Average velocity = 3.556(Ft/s)
Total irregular channel flow = 13.047(CFS)
Irregular channel normal depth above invert elev. = 0.606(Ft.)
Average velocity of channel(s) = 3.556(Ft/s)
Process from Point/Station 12.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm)=
                                            0.840(In/Hr)
Time of concentration = 19.74 min.
Rainfall intensity = 3.070(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.654
Subarea runoff = 26.499(CFS) for 14.500(Ac.)
               39.547(CFS)
Total runoff =
Effective area this stream =
                            19.70(Ac.)
Total Study Area (Main Stream No. 1) = 19.70(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 13.000 to Point/Station 14.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 0.560(Ft.), Average velocity = 4.524(Ft/s)
   ****** Irregular Channel Data *******
______
Information entered for subchannel number 1:
              'X' coordinate
Point number
                              'Y' coordinate
                0.00
                                1.00
     1
                10.00
                                0.00
                20.00
                                0.00
                30.00
                                1.00
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```
Manning's 'N' friction factor = 0.035
Sub-Channel flow = 39.547(CFS)
 ' flow top width = 21.205(Ft.)
          velocity= 4.524(Ft/s)
      ' area = 8.741(Sq.Ft)
      ' Froude number = 1.242
Upstream point elevation = 1976.000(Ft.)
Downstream point elevation = 1938.000(Ft.)
Flow length = 1023.000(Ft.)
Travel time = 3.77 min.
Time of concentration = 23.51 min.
Depth of flow = 0.560(Ft.)
Average velocity = 4.524(Ft/s)
Total irregular channel flow = 39.547(CFS)
Irregular channel normal depth above invert elev. = 0.560(Ft.)
Average velocity of channel(s) = 4.524(Ft/s)
Process from Point/Station 13.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm)=
                                               0.840(In/Hr)
Time of concentration = 23.51 min.

Rainfall intensity = 2.717(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.622
Subarea runoff = 48.805(CFS) for 32.600(Ac.)
Total runoff = 88.352(CFS)
Effective area this stream =
                             52.30(Ac.)
Total Study Area (Main Stream No. 1) = 52.30(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 14.000 to Point/Station 15.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 1.308(Ft.), Average velocity = 4.083(Ft/s)
  ****** Irregular Channel Data ********
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
                                 2.00
     1
                 0.00
     2
                 10.00
                                 0.00
     3
                 20.00
                                  0.00
                 30.00
                                 2.00
Manning's 'N' friction factor = 0.035
Sub-Channel flow = 88.352(CFS)
 ' flow top width = 23.082(Ft.)
' velocity= 4.083(Ft/s)
  ' area = 21.638(Sq.Ft)
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' Froude number = 0.743
Upstream point elevation = 1938.000(Ft.)
Downstream point elevation = 1919.000(Ft.)
Flow length = 1857.000(Ft.)
Travel time = 7.58 min.
Time of concentration = 31.09 min.
Depth of flow = 1.308(Ft.)
Average velocity = 4.083(Ft/s)
Total irregular channel flow = 88.352(CFS)
Irregular channel normal depth above invert elev. = 1.308(Ft.)
Average velocity of channel(s) = 4.083(Ft/s)
Process from Point/Station 14.000 to Point/Station 15.000
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Time of concentration = 31.09 min.
Rainfall intensity = 2.234(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(O=KCIA) is C = 0.562
Subarea runoff = 31.109(CFS) for 42.900(Ac.)
Total runoff = 119.460(CFS)
Effective area this stream =
                            95.20(Ac.)
Total Study Area (Main Stream No. 1) = 95.20(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 15.000 to Point/Station 16.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 2.323(Ft.), Average velocity = 3.095(Ft/s)
   ****** Irregular Channel Data *******
   _____
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
                0.00
                                3.00
     1
     2
                15.00
                                0.00
     3
                20.00
                                0.00
                35.00
                                3.00
Manning's 'N' friction factor = 0.035
______
Sub-Channel flow = 119.460(CFS)
 ' ' flow top width = 28.229(Ft.)
    velocity= 3.095(Ft/s)
area = 38.594(Sq.Ft)
Froude number = 0.467
Upstream point elevation = 1919.000(Ft.)
Downstream point elevation = 1909.000(Ft.)
Flow length = 2794.000(Ft.)
Travel time = 15.04 min.
```

```
Time of concentration = 46.13 min.
Depth of flow = 2.323(Ft.)
Average velocity = 3.095(Ft/s)
Total irregular channel flow = 119.460(CFS)
Irregular channel normal depth above invert elev. = 2.323(Ft.)
Average velocity of channel(s) = 3.095(Ft/s)
Process from Point/Station 15.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
0.840(In/Hr)
Time of concentration = \frac{46.13 \text{ min.}}{1.695(\text{In/Hr})} for a
                                           100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.454
Subarea runoff = 5.804(CFS) for 67.600(Ac.)
Total runoff = 125.265(CFS)Q_{100}
                              162.80(Ac.)
Effective area this stream =
Total Study Area (Main Stream No. 1) =
                                       162.80(Ac.)
Area averaged Fm value =
                         0.840(In/Hr)
End of computations, Total Study Area =
                                            162.80 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Note: These figures do not consider reduced effective area
effects caused by confluences in the rational equation.
Area averaged pervious area fraction(Ap) = 1.000
```

Area averaged SCS curve number = 67.0

San Bernardino County Rational Hydrology Program

(Hydrology Manual Date - August 1986)

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CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989-2004 Version 7.0
    Rational Hydrology Study Date: 06/05/23
______
REIDO FARMS, LLC - JOB 3582.001
OFF-SITE TRIBUTARY STORM RUNOFF
NODE 21 - NODE 26
100-YEAR STORM EVENT - AMC 1
______
MERRELL JOHNSON COMPANIES
22221 HIGHWAY 18
APPLE VALLEY, CA 92307
(760) 240-8000 * FAX (760) 240-1400
______
******* Hydrology Study Control Information ********
_____
Rational hydrology study storm event year is 100.0
Computed rainfall intensity:
                                   1.410 (In.)
Storm year = 100.00 1 hour rainfall =
Slope used for rainfall intensity curve b = 0.7000
Soil antecedent moisture condition (AMC) = 1
Process from Point/Station 21.000 to Point/Station
**** INITIAL AREA EVALUATION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Initial subarea data:
Initial area flow distance = 875.000(Ft.)
Top (of initial area) elevation = 2049.000(Ft.)
Bottom (of initial area) elevation = 2024.000(Ft.)
Difference in elevation = 25.000(Ft.)
Slope = 0.02857 s(%) =
                      2.86
TC = k(0.525)*[(length^3)/(elevation change)]^0.2
Initial area time of concentration = 16.061 min.
Rainfall intensity = 3.547(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area (Q=KCIA) is C = 0.687
Subarea runoff = 14.863(CFS)
Total initial stream area =
                           6.100(Ac.)
Pervious area fraction = 1.000
Initial area Fm value =
                    0.840(In/Hr)
Process from Point/Station 22.000 to Point/Station 23.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 0.364(Ft.), Average velocity = 3.458(Ft/s)
    ****** Irregular Channel Data *******
```

```
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
     1
                0.00
                                2.00
     2
                10.00
                                0.00
     3
                20.00
                                0.00
                30.00
                                2.00
Manning's 'N' friction factor = 0.035
______
Sub-Channel flow = 14.863(CFS)
 flow top width = 13.637(Ft.)
velocity= 3.458(Ft/s)
area = 4.298(Sq.Ft)
      ' Froude number = 1.085
Upstream point elevation = 2024.000(Ft.)
Downstream point elevation = 1996.000(Ft.)
Flow length = 899.000(Ft.)
Travel time = 4.33 \text{ min.}
Time of concentration = 20.39 min.
Depth of flow = 0.364(Ft.)
Average velocity = 3.458(Ft/s)
Total irregular channel flow = 14.863(CFS)
Irregular channel normal depth above invert elev. = 0.364(Ft.)
Average velocity of channel(s) = 3.458(Ft/s)
Process from Point/Station 22.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Time of concentration = 20.39 min.
Rainfall intensity = 3.001(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.648
Subarea runoff = 33.569(CFS) for 18.800(Ac.)
Total runoff =
               48.432(CFS)
Effective area this stream =
                            24.90(Ac.)
Total Study Area (Main Stream No. 1) = 24.90(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 23.000 to Point/Station 24.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 0.671(Ft.), Average velocity = 5.402(Ft/s)
   ****** Irregular Channel Data *******
_____
Information entered for subchannel number 1:
              'X' coordinate
Point number
                              'Y' coordinate
     1
                0.00
                                2.00
                10.00
                                0.00
                20.00
                                0.00
                30.00
                                2.00
```

```
Manning's 'N' friction factor = 0.035
Sub-Channel flow = 48.432(CFS)
 ' flow top width = 16.712(Ft.)
          velocity= 5.402(Ft/s)
      ' area = 8.965(Sq.Ft)
      ' Froude number = 1.300
Upstream point elevation = 1996.000(Ft.)
Downstream point elevation = 1957.000(Ft.)
Flow length = 1039.000(Ft.)
Travel time = 3.21 min.
Time of concentration = 23.60 min.
Depth of flow = 0.671(Ft.)
Average velocity = 5.402(Ft/s)
Total irregular channel flow = 48.432(CFS)
Irregular channel normal depth above invert elev. = 0.671(Ft.)
Average velocity of channel(s) = 5.402(Ft/s)
Process from Point/Station 23.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm)=
                                              0.840(In/Hr)
Time of concentration = 23.60 min.

Rainfall intensity = 2.710(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.621
Subarea runoff = 56.567(CFS) for 37.500(Ac.)
Total runoff = 104.999(CFS)
Effective area this stream =
                             62.40(Ac.)
Total Study Area (Main Stream No. 1) = 62.40(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 24.000 to Point/Station 25.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 0.933(Ft.), Average velocity = 5.726(Ft/s)
  ****** Irregular Channel Data *******
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
     1
                 0.00
                                 3.00
     2
                 15.00
                                 0.00
     3
                 30.00
                                  0.00
                 45.00
                                 3.00
Manning's 'N' friction factor = 0.035
Sub-Channel flow = 104.999(CFS)
 ' flow top width = 24.326(Ft.)
' velocity= 5.726(Ft/s)
  ' area = 18.338(Sq.Ft)
```

```
' Froude number = 1.162
Upstream point elevation = 1957.000(Ft.)
Downstream point elevation = 1925.000(Ft.)
Flow length = 1195.000(Ft.)
Travel time = 3.48 min.
Time of concentration = 27.08 min.
Depth of flow = 0.933(Ft.)
Average velocity = 5.726(Ft/s)
Total irregular channel flow = 104.999(CFS)
Irregular channel normal depth above invert elev. = 0.933(Ft.)
Average velocity of channel(s) = 5.726(Ft/s)
Process from Point/Station 24.000 to Point/Station 25.000
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Time of concentration = 27.08 min.
Rainfall intensity = 2.461(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(O=KCIA) is C = 0.593
Subarea runoff = 78.533(CFS) for 63.400(Ac.)
Total runoff = 183.531(CFS)
                           125.80(Ac.)
Effective area this stream =
Total Study Area (Main Stream No. 1) = 125.80(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 25.000 to Point/Station 26.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 1.729(Ft.), Average velocity = 3.705(Ft/s)
   ****** Irregular Channel Data *******
   ______
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
                0.00
                                3.00
     1
     2
                15.00
                                0.00
     3
                35.00
                                0.00
                50.00
                                3.00
Manning's 'N' friction factor = 0.035
______
Sub-Channel flow = 183.531(CFS)
 ' ' flow top width = 37.293(Ft.)
    velocity= 3.705(Ft/s)
area = 49.537(Sq.Ft)
Froude number = 0.566
Upstream point elevation = 1925.000(Ft.)
Downstream point elevation = 1909.000(Ft.)
Flow length = 3031.000(Ft.)
Travel time = 13.64 min.
```

```
Time of concentration = 40.71 \text{ min.}
Depth of flow = 1.729(Ft.)
Average velocity = 3.705(Ft/s)
Total irregular channel flow = 183.531(CFS)
Irregular channel normal depth above invert elev. = 1.729(Ft.)
Average velocity of channel(s) = 3.705(Ft/s)
Process from Point/Station 25.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
The area added to the existing stream causes a
a lower flow rate of Q = 179.502(CFS)
therefore the upstream flow rate of Q =
                                         183.531(CFS) is being used
Time of concentration = 40.71 min.T<sub>c</sub>

Rainfall intensity = 1.850(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.491
Subarea runoff = 0.000(CFS) for 71.700(Ac.)
Total runoff = 183.531(CFS) Q_{100}
                                197.50(Ac.)
Effective area this stream =
Total Study Area (Main Stream No. 1) = 197.50(Ac.)
Area averaged Fm value = 0.840(In/Hr)
End of computations, Total Study Area =
                                               197.50 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Note: These figures do not consider reduced effective area
effects caused by confluences in the rational equation.
Area averaged pervious area fraction(Ap) = 1.000
```

Area averaged SCS curve number = 67.0

San Bernardino County Rational Hydrology Program

(Hydrology Manual Date - August 1986)

```
CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989-2004 Version 7.0
    Rational Hydrology Study Date: 06/05/23
______
REIDO FARMS, LLC - JOB 3258.001
OFF-SITE TRIBUTARY STORM RUNOFF FLOW
NODE 31 - NODE 36
100-YEAR STORM EVENT - AMC I
_____
MERRELL JOHNSON COMPANIES
22221 HIGHWAY 18
APPLE VALLEY, CA 92307
(760) 240-8000 * FAX (760) 240-1400
______
******* Hydrology Study Control Information ********
______
Rational hydrology study storm event year is 100.0
Computed rainfall intensity:
                                   1.410 (In.)
Storm year = 100.00 1 hour rainfall =
Slope used for rainfall intensity curve b = 0.7000
Soil antecedent moisture condition (AMC) = 1
Process from Point/Station 31.000 to Point/Station
**** INITIAL AREA EVALUATION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Initial subarea data:
Initial area flow distance = 997.000(Ft.)
Top (of initial area) elevation = 1977.000(Ft.)
Bottom (of initial area) elevation = 1942.000(Ft.)
Difference in elevation = 35.000(Ft.)
Slope = 0.03511 s(%) =
                      3.51
TC = k(0.525)*[(length^3)/(elevation change)]^0.2
Initial area time of concentration = 16.239 min.
Rainfall intensity = 3.520(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area (Q=KCIA) is C = 0.685
Subarea runoff = 23.879(CFS)
Total initial stream area =
                           9.900(Ac.)
Pervious area fraction = 1.000
Initial area Fm value =
                    0.840(In/Hr)
Process from Point/Station 32.000 to Point/Station 33.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 0.634(Ft.), Average velocity = 3.321(Ft/s)
    ****** Irregular Channel Data *******
```

```
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
     1
                0.00
                                1.00
     2
                10.00
                                0.00
     3
                15.00
                                0.00
                25.00
                                 1.00
Manning's 'N' friction factor = 0.035
______
Sub-Channel flow = 23.879(CFS)
 flow top width = 17.680(Ft.)
velocity= 3.321(Ft/s)
area = 7.190(Sq.Ft)
      ' Froude number = 0.918
Upstream point elevation = 1942.000(Ft.)
Downstream point elevation = 1926.000(Ft.)
Flow length = 784.000(Ft.)
Travel time = 3.93 min.
Time of concentration = 20.17 min.
Depth of flow = 0.634(Ft.)
Average velocity = 3.321(Ft/s)
Total irregular channel flow = 23.879(CFS)
Irregular channel normal depth above invert elev. = 0.634(Ft.)
Average velocity of channel(s) = 3.321(Ft/s)
Process from Point/Station 32.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Time of concentration = 20.17 min.
Rainfall intensity = 3.024(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.650
Subarea runoff = 38.630(CFS) for
                                 21.900(Ac.)
Total runoff =
               62.509(CFS)
Effective area this stream =
                             31.80(Ac.)
Total Study Area (Main Stream No. 1) = 31.80(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 33.000 to Point/Station 34.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 1.105(Ft.), Average velocity = 3.643(Ft/s)
   ****** Irregular Channel Data *******
_____
Information entered for subchannel number 1:
              'X' coordinate
Point number
                              'Y' coordinate
     1
                0.00
                                2.00
                10.00
                                0.00
                20.00
                                0.00
                30.00
                                 2.00
```

```
Manning's 'N' friction factor = 0.035
Sub-Channel flow = 62.509(CFS)
 ' flow top width = 21.052(Ft.)
          velocity= 3.643(Ft/s)
      ' area = 17.159(Sq.Ft)
      ' Froude number = 0.711
Upstream point elevation = 1926.000(Ft.)
Downstream point elevation = 1916.000(Ft.)
Flow length = 1020.000(Ft.)
Travel time = 4.67 min.
Time of concentration = 24.84 min.
Depth of flow = 1.105(Ft.)
Average velocity = 3.643(Ft/s)
Total irregular channel flow = 62.509(CFS)
Irregular channel normal depth above invert elev. = 1.105(Ft.)
Average velocity of channel(s) = 3.643(Ft/s)
Process from Point/Station 33.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm)=
                                               0.840(In/Hr)
Time of concentration = 24.84 \text{ min.}
Rainfall intensity = 2.614(\text{In/Hr}) \text{ for a} 100.0 \text{ year storm}
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.611
Subarea runoff = 61.880(CFS) for 46.100(Ac.)
Total runoff = 124.389(CFS)
Effective area this stream =
                              77.90(Ac.)
Total Study Area (Main Stream No. 1) = 77.90(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 34.000 to Point/Station 35.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 1.640(Ft.), Average velocity = 3.269(Ft/s)
  ****** Irregular Channel Data *******
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
     1
                 0.00
                                  3.00
     2
                 15.00
                                  0.00
     3
                 30.00
                                  0.00
                 45.00
                                  3.00
Manning's 'N' friction factor = 0.035
Sub-Channel flow = 124.389(CFS)
 flow top width = 31.402(Ft.)
velocity= 3.269(Ft/s)
  ' area = 38.054(Sq.Ft)
```

```
' Froude number = 0.523
Upstream point elevation = 1916.000(Ft.)
Downstream point elevation = 1909.000(Ft.)
Flow length = 1505.000(Ft.)
Travel time = 7.67 min.
Time of concentration = 32.51 min.
Depth of flow = 1.640(Ft.)
Average velocity = 3.269(Ft/s)
Total irregular channel flow = 124.389(CFS)
Irregular channel normal depth above invert elev. = 1.640(Ft.)
Average velocity of channel(s) = 3.269(Ft/s)
Process from Point/Station 34.000 to Point/Station 35.000
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
Pervious ratio(Ap) = 1.0000 Max loss rate(Fm) = 0.840(In/Hr)
Time of concentration = 32.51 min.
Rainfall intensity = 2.165(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.551
Subarea runoff = 71.452(CFS) for 86.300(Ac.)
Total runoff = 195.842(CFS)
                           164.20(Ac.)
Effective area this stream =
Total Study Area (Main Stream No. 1) = 164.20(Ac.)
Area averaged Fm value = 0.840(In/Hr)
Process from Point/Station 35.000 to Point/Station 36.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****
Depth of flow = 1.798(Ft.), Average velocity = 3.759(Ft/s)
   ****** Irregular Channel Data *******
   _____
Information entered for subchannel number 1:
Point number 'X' coordinate 'Y' coordinate
                0.00
                                3.00
     1
     2
                15.00
                                0.00
     3
                35.00
                                0.00
                                3.00
                50.00
Manning's 'N' friction factor = 0.035
______
Sub-Channel flow = 195.842(CFS)
 ' ' flow top width = 37.975(Ft.)
    velocity= 3.759(Ft/s)
area = 52.105(Sq.Ft)
Froude number = 0.565
Upstream point elevation = 1909.000(Ft.)
Downstream point elevation = 1902.000(Ft.)
Flow length = 1345.000(Ft.)
Travel time = 5.96 min.
```

```
Time of concentration = 38.48 min.
Depth of flow = 1.798(Ft.)
Average velocity = 3.759(Ft/s)
Total irregular channel flow = 195.842(CFS)
Irregular channel normal depth above invert elev. = 1.798(Ft.)
Average velocity of channel(s) = 3.759(Ft/s)
Process from Point/Station 35.000 to Point/Station
**** SUBAREA FLOW ADDITION ****
UNDEVELOPED (poor cover) subarea
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
SCS curve number for soil(AMC 2) = 67.00
Adjusted SCS curve number for AMC 1 = 47.40
0.840(In/Hr)
Time of concentration = 38.48 min. T<sub>c</sub>
Rainfall intensity = 1.924(In/Hr) for a
                                              100.0 year storm
Effective runoff coefficient used for area, (total area with modified
rational method)(Q=KCIA) is C = 0.507
Subarea runoii = 285.187(CFS) Q<sub>100</sub>
Total runoff = 285.187(CFS) Q<sub>100</sub>
+his stream = 292.20(Ac.)
Subarea runoff = 89.345(CFS) for 128.000(Ac.)
Total Study Area (Main Stream No. 1) =
                                         292.20(Ac.)
Area averaged Fm value =
                          0.840(In/Hr)
End of computations, Total Study Area =
                                                292.20 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Note: These figures do not consider reduced effective area
effects caused by confluences in the rational equation.
Area averaged pervious area fraction(Ap) = 1.000
```

Area averaged SCS curve number = 67.0

UNIT HYDROGRAPH CALCULATIONS: 100-YEAR STORM

Unit Hydrograph Analysis

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Study date 06/04/23

+++++++++++++++++++++++++++++++++++++++						
San Bernardino County Synthetic Unit Hydrology Method Manual date - August 1986 MERRELL JOHNSON COMPANIES 22221 HIGHWAY 18 APPLE VALLEY, CA 92307 (760) 240-8000 * FAX (760) 240-1400						
REIDO FARMS, LLC - JOB 3852.001 OFF-SITE STORM RUNOFF UNIT HYDROGRAPH - SOUTH OF INTERSTATE 40 100-YEAR STORM EVENT - AMC I						
Storm Event Year = 100						
Antecedent Moisture Condition = 1						
English (in-lb) Input Units Used						
English Rainfall Data (Inches) Input Values Used						
English Units used in output format						
Area averaged rainfall intensity isohyetal data: Sub-Area Duration Isohyetal (Ac.) (hours) (In) Rainfall data for year 10						
8260.00 1 0.77						
Rainfall data for year 2 8260.00 6 0.83						
Rainfall data for year 2 8260.00 24 1.29						

Rainfall data for year 100

```
1 1.40
       8260.00
______
Rainfall data for year 100
               6 2.26
       8260.00
Rainfall data for year 100
                         3.45
       8260.00 24
****** Area-averaged max loss rate, Fm ******
SCS curve SCS curve
                   Area
                         Area
                                 Fp(Fig C6) Ap
Area-averaged adjusted loss rate Fm (In/Hr) = 0.373
****** Area-Averaged low loss rate fraction, Yb *******
                  SCS CN
Area
        Area
                           SCS CN
                                        Pervious
(Ac.)
       Fract
                  (AMC2)
                           (AMC1)
                                        Yield Fr
 8260.00 1.000
                   91.0
                           79.8 2.53 0.459
Area-averaged catchment yield fraction, Y = 0.459
Area-averaged low loss fraction, Yb = 0.541
Watercourse length = 45326.00(Ft.)
Length from concentration point to centroid = 23512.00(Ft.)
Elevation difference along watercourse = 3152.00(Ft.)
Mannings friction factor along watercourse = 0.040
Watershed area =
               8260.00(Ac.)
Catchment Lag time =
                 1.248 hours
Unit interval = 10.000 minutes
Unit interval percentage of lag time = 13.3537
Hydrograph baseflow = 0.00(CFS)
Average maximum watershed loss rate(Fm) = 0.373(In/Hr)
Average low loss rate fraction (Yb) = 0.541 (decimal)
DESERT S-Graph Selected
Computed peak 5-minute rainfall = 0.664(In)
Computed peak 30-minute rainfall = 1.137(In)
Specified peak 1-hour rainfall = 1.400(In)
Computed peak 3-hour rainfall = 1.878(In)
Specified peak 6-hour rainfall = 2.260(In)
Specified peak 24-hour rainfall = 3.450(In)
Rainfall depth area reduction factors:
Using a total area of 8260.00(Ac.) (Ref: fig. E-4)
```

Interval 'S' Graph Unit Hydrograph
Number Mean values ((CFS))

(K = 49947.19 (CFS))1 0.590 294.563 2 2.374 891.122 3 5.301 1461.984 4 9.537 2115.669 5 17.221 3837.958 6 29.543 6154.718 7 41.111 5777.967 8 49.888 4383.861 9 56.723 3413.657 10 61.818 2544.835 11 65.888 2033.048 12 69.348 1727.818 13 72.377 1512.948 14 74.927 1273.647 15 77.230 1150.170 16 79.222 995.076 17 80.981 878.596 18 82.539 778.060 19 83.975 717.233 20 85.298 661.126 21 86.563 631.485 22 87.668 552.379 23 88.699 514.538 414.649 24 89.529 25 90.318 394.339 26 91.069 375.032 27 91.784 356.854 28 92.427 321.327 29 93.045 308.910 30 93.633 293.492 31 94.148 257.135 32 94.602 226.774 33 226.774 95.056 34 95.505 224.347

35		95.878			186.429		
36		96.226			173.415		
37		96.573			173.415		
38		96.903			165.129		
39		97.154			125.285		
40		97.395			120.057		
41		97.635			120.057		
42		97.840			102.420		
43		97.975			67.631		
43		98.109			66.698		
44 45							
		98.242			66.730		
46		98.391			74.254		
47		98.551			80.038		
48		98.712			80.038		
49		98.872			80.038		
50		99.032			80.038		
51		99.192			80.038		
52		99.353			80.038		
53		99.500			73.582		
54		99.590			45.065		
55		99.674			41.686		
56		99.757			41.686		
57		99.841			41.686		
58		99.924			41.686		
59		100.000			37.959		
Tot	al soil rain 🛚	loss =		1.58(In)			
Tot	al effective	rainfall	=	1.83(In)			
Pea	k flow rate i	n flood	hydr	ograph = 46	51.61(CFS)		
+++	++++++++++++	++++++	++++	+++++++++++	++++++++	++++++++	++++++
		24 -	Н О	UR STO	RM		
	R	unof	f	Hydro	graph		
	Hydro	ograph i	n 1	0 Minute in	tervals ((CFS))	
	•	0 1			``	,,	
Time(h+m)	Volume Ac.Ft	O(CFS) 0	1175.0	2350.0	3525.0	4700.0
0+10	0.0138	1.00	0	1	1	I	1
0+20	0.0695			i	i	i	i
0+30	0.1942		-	i	i	i	i
0+40	0.4189	16.32	-	ľ	i	i	
0+40 0+50	0.8253	29.50	_	l I			
1+ 0	1.5233	50.67		l I		-	
1+ 0			_	l I	ł		
	2.4974	70.72	Q	 	!		l I
1+20	3.6842	86.16	Q	l I	ļ	ļ	
1+30	5.0398	98.42	Q	I	I	I	I

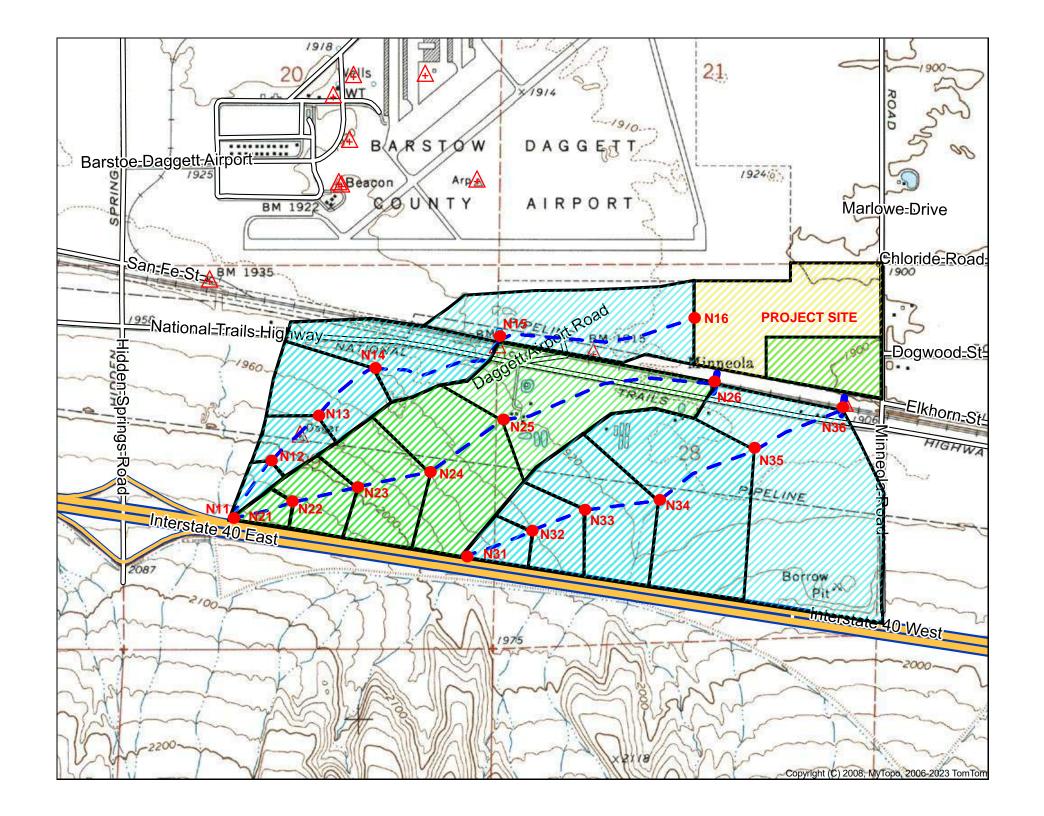
1+40 6.5250 107.82 Q								
1+50 8.1166 115.55 Q	1+40	6.5250	107.82	Q		- 1	1	
2+ 0	1+50	8.1166	115.55		Ì	Ì	İ	İ
2+10 11.5699 128.40 VQ	2+ 0	9.8013	122.31		į	į	İ	j
2+20	2+10	11.5699	128.40	-	į	į	İ	į
2+30				_	i	i	i	i
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3+30				_	i	i	i	i
3+40					i	i	i	
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9+ 0 122.8990 255.77 QV				_		ļ	1	ļ
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9±40 137 5045 270 93 1 0 V 1 I I I I				-	ļ	ļ	ļ	!
: · · · · · · · · · · · · · · · · · · ·	9+40	137.5045	270.93	Q V		ļ		ļ
9+50 141.2931 275.05 Q V	9+50	141.2931	275.05	Q V			1	

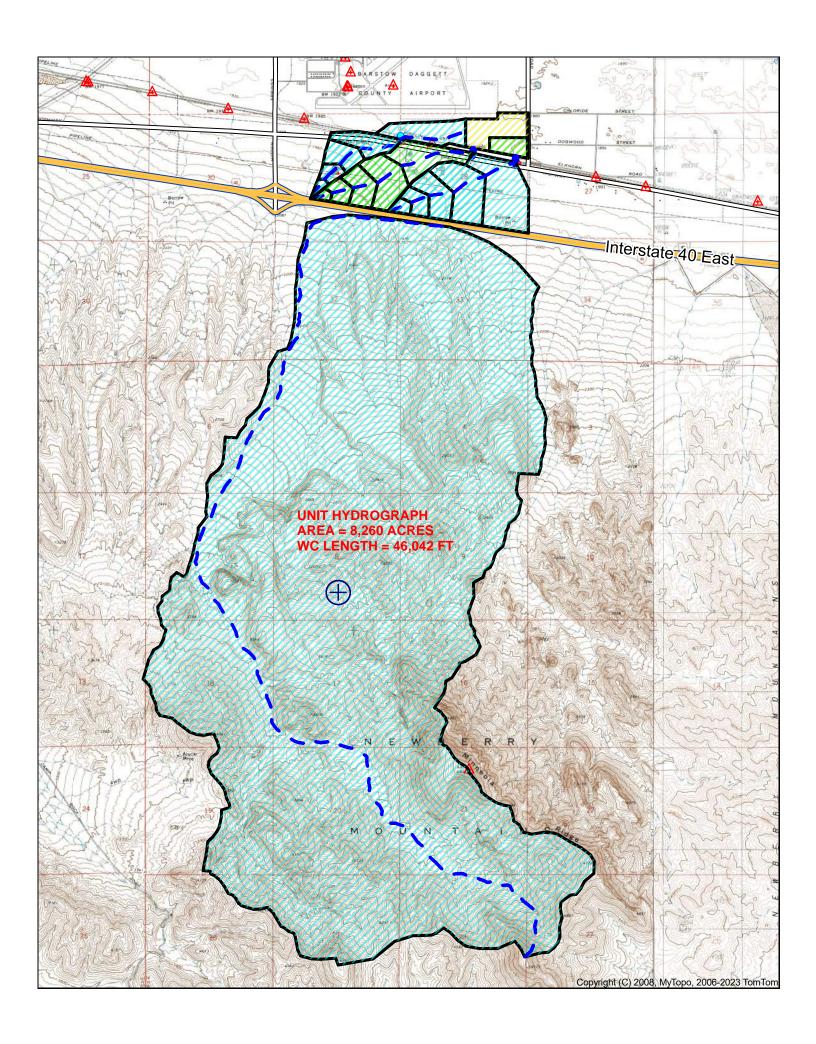
10+ 0	145.1386	279.19	Q V
10+10	149.0433	283.48	Q V
10+20	153.0096	287.95	Q V
10+30	157.0398	292.60	Q V
10+40	161.1368	297.44	Q V
10+50	165.3032	302.48	Q V
11+ 0	169.5421	307.75	Q V
11+10	173.8569	313.25	Q V
11+20	178.2510	319.01	Q V
11+30	182.7282	325.04	Q V
11+40	187.2926	331.37	Q V
11+50	191.9486	338.02	Q V
12+ 0	196.7010	345.02	Q V
12+10	201.5537	352.31	Q V
12+20	206.5101	359.83	Q V
12+30	211.5737	367.62	Q V
12+40	216.7486	375.70	Q V
12+50	222.0350	383.79	Q V
13+ 0	227.4310	391.76	Q V
13+10	232.9473	400.48	Q V
13+20	238.5995	410.35	Q V
13+30	244.4031	421.34	Q V
13+40	250.3746	433.53	Q V
13+50	256.5308	446.94	Q V
14+ 0	262.8900	461.68	Q V
14+10	269.5148	480.96	Q V
14+20	276.5150	508.22	Q V
14+30	284.0019	543.55	Q V
14+40	292.1048	588.27	Q V
14+50	301.1141	654.08	Q V
15+ 0	311.4178	748.05	Q V
15+10	323.0344	843.37	Q V
15+20	335.8526	930.60	Q V
15+30	349.7721	1010.56	Q V
15+40	364.6683	1081.47	Q V
15+50	380.5404	1152.31	Q V
16+ 0	397.7725	1251.05	Q V
16+10	418.2353	1485.60	QV
16+20	443.2910	1819.05	VQ
16+30	473.1986	2171.29	V Q
16+40	510.0579	2675.99	V Q
16+50	560.5685	3667.07	V Q
17+ 0	624.6403	4651.61	Q ₁₀₀ V Q
17+10	684.7644	4365.01	V Q
17+20	735.2381	3664.39	V Q
17+30	778.3327	3128.67	V Q
17+40	814.7344	2642.77	Q V
17+50	846.4923	2305.62	Q V
18+ 0	874.5898	2039.88	Q V
18+10	899.6538	1819.65	Q V

18+20	921.9431	1618.20	Q V	
18+30	942.3112	1478.73	i i g i vi i	
18+40	960.7813	1340.93		
18+50	977.7313	1230.57	į Q į v į	
19+ 0	993.3988	1137.46	i qi i ıv i	
19+10	1008.0847	1066.19	j čj j v j	
19+20	1021.9084	1003.61		
19+30	1035.0077	951.01	į gi į v	
19+40	1047.1680	882.84		
19+50	1058.6013	830.05	į į į v į	
20+ 0	1069.0634	759.55		
20+10	1079.0576	725.58		
20+20	1088.6030	693.00		
20+30	1097.7091	661.10	į į į v į	
20+40	1106.3272	625.67	į į į v į	
20+50	1114.6024	600.78	į į į v į	
21+ 0	1122.5091	574.03		
21+10	1129.9481	540.07	į į į v į	
21+20	1137.0033	512.21	į į į v į	
21+30	1143.8541	497.37	į į į v į	
21+40	1150.4568	479.36	į į į v į	
21+50	1156.6503	449.65		
22+ 0	1162.6060	432.39	į į į v į	
22+10	1168.3927	420.11	į į į v į	
22+20	1173.9389	402.66		
22+30	1179.1124	375.60	į į į v į	
22+40	1184.1285	364.17		
22+50	1188.9997	353.65	Q V	
23+ 0	1193.6067	334.47	Q	
23+10	1197.8946	311.31	Q	
23+20	1202.0744	303.45	Q	
23+30	1206.1685	297.24	Q	
23+40	1210.2258	294.56	Q	
23+50	1214.2318	290.84	Q	
24+ 0	1218.1521	284.62	Q	
24+10	1221.9808	277.96	Q	
24+20	1225.6964	269.75	Q	
24+30	1229.2712	259.53	Q	
24+40	1232.6632	246.26	Q	
24+50	1235.7303	222.68	Q	
25+ 0	1238.2674	184.19	Q	
25+10	1240.4514	158.56	Q	
25+20	1242.3772	139.81	Q	
25+30	1244.0923	124.52	Q	
25+40	1245.6316	111.75	Q V V	
25+50	1246.9632	96.68	Q V V	
26+ 0	1247.9511	71.72	Q V V	
26+10	1248.8364	64.27	Q V V	
26+20	1249.6291	57.55	Q V	
26+30	1250.3399	51.61	Q V	

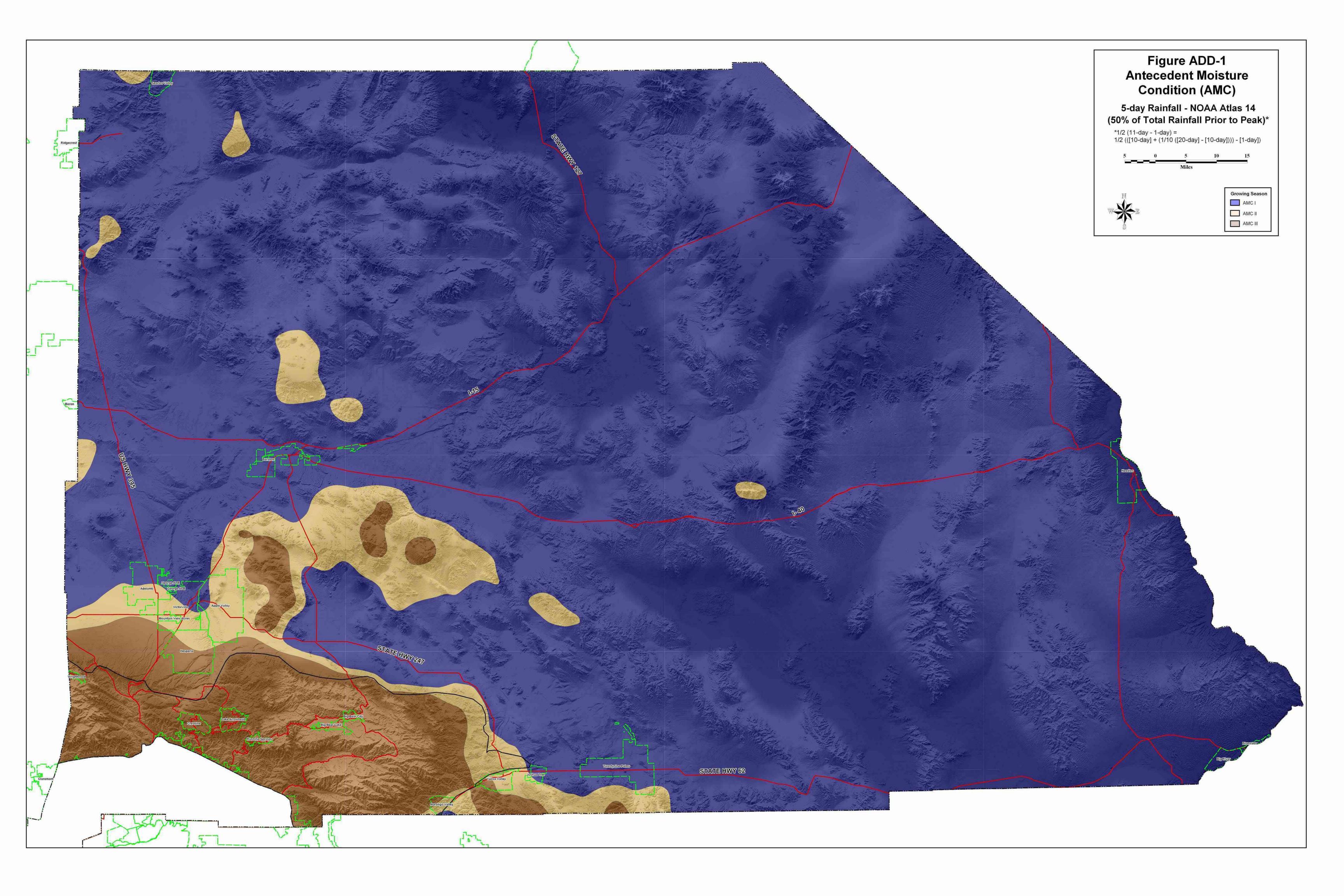
26+40	1250.9796	46.44	Q
26+50	1251.5568	41.91	Q
27+ 0	1252.0836	38.25	Q
27+10	1252.5645	34.91	Q
27+10	1253.0035	31.87	Q
27+30	1253.4032	29.02	Q
27+30	1253.7684	26.51	
			Q
27+50	1254.1018	24.20	Q
28+ 0	1254.4086	22.28	Q
28+10	1254.6906	20.47	Q
28+20	1254.9491	18.77	Q
28+30	1255.1858	17.18	Q
28+40	1255.4027	15.75	Q
28+50	1255.6008	14.39	Q
29+ 0	1255.7814	13.11	Q
29+10	1255.9465	11.99	Q
29+20	1256.0978	10.99	Q
29+30	1256.2358	10.01	Q
29+40	1256.3606	9.07	Q
29+50	1256.4745	8.27	Q
30+ 0	1256.5782	7.53	Q
30+10	1256.6719	6.81	Q
30+20	1256.7563	6.12	
			Q
30+30	1256.8333	5.59	Q
30+40	1256.9034	5.09	Q
30+50	1256.9666	4.59	Q
31+ 0	1257.0240	4.17	Q
31+10	1257.0773	3.87	Q
31+20	1257.1265	3.58	Q
31+30	1257.1718	3.29	Q
31+40	1257.2130	2.99	Q
31+50	1257.2497	2.67	Q
32+ 0	1257.2822	2.36	Q
32+10	1257.3104	2.05	Q
32+20	1257.3344	1.75	Q
32+30	1257.3544	1.45	Q
32+40	1257.3703	1.16	Q
32+50	1257.3825	0.89	Q
33+ 0	1257.3925	0.89	
			Q
33+10	1257.4003	0.57	Q
33+20	1257.4061	0.42	Q
33+30	1257.4099	0.28	Q
33+40	1257.4117	0.13	Q

TRIBUTARY DRAINAGE MAP





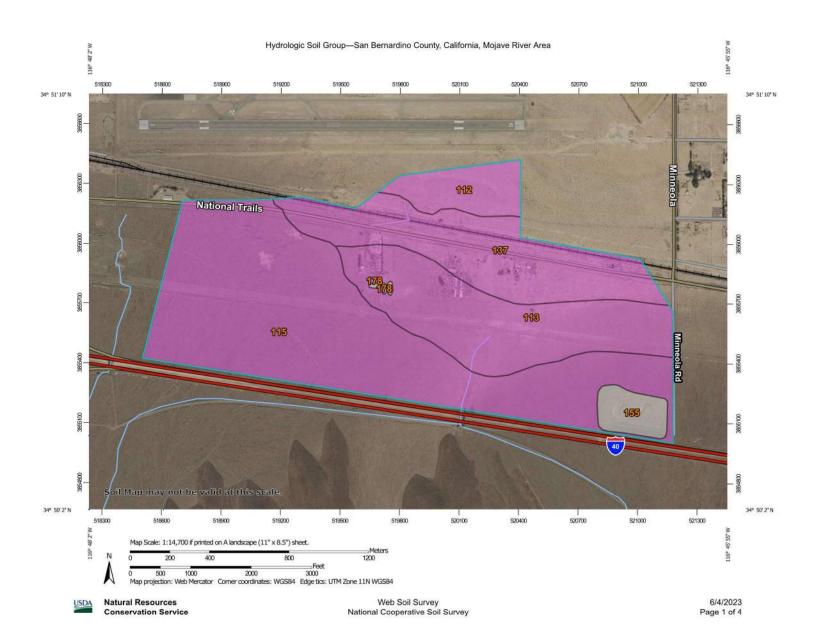






EXHIBITS

SOILS MAP



MAP LEGEND **MAP INFORMATION** The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) C . 1:24,000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available 10 Α misunderstanding of the detail of mapping and accuracy of soil Water Features line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed В scale. Transportation B/D Rails +++ Please rely on the bar scale on each map sheet for map С Interstate Highways C/D Source of Map: Natural Resources Conservation Service US Routes Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Soil Rating Lines Background Aerial Photography Albers equal-area conic projection, should be used if more A/D accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: San Bernardino County, California, Mojave C/D Survey Area Data: Version 14, Sep 1, 2022 Soil map units are labeled (as space allows) for map scales Not rated or not available Date(s) aerial images were photographed: Feb 27, 2021—May Soil Rating Points 27. 2021 A The orthophoto or other base map on which the soil lines were A/D compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. В B/D

Hydrologic Soil Group

	_			
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
112	CAJON SAND, 0 TO 2 PERCENT SLOPES	А	35.0	5.6%
113	CAJON SAND, 2 TO 9 PERCENT SLOPES	А	133.3	21.5%
115	CAJON GRAVELLY SAND, 2 TO 15 PERCENT SLOPES	A	308.9	49.8%
137	KIMBERLINA LOAMY FINE SAND, COOL, 0 TO 2 PERCENT SLOPES	A	124.0	20.0%
155	PITS		18.5	3.0%
178	WATER		0.9	0.1%
Totals for Area of Inter	est	620.7	100.0%	

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

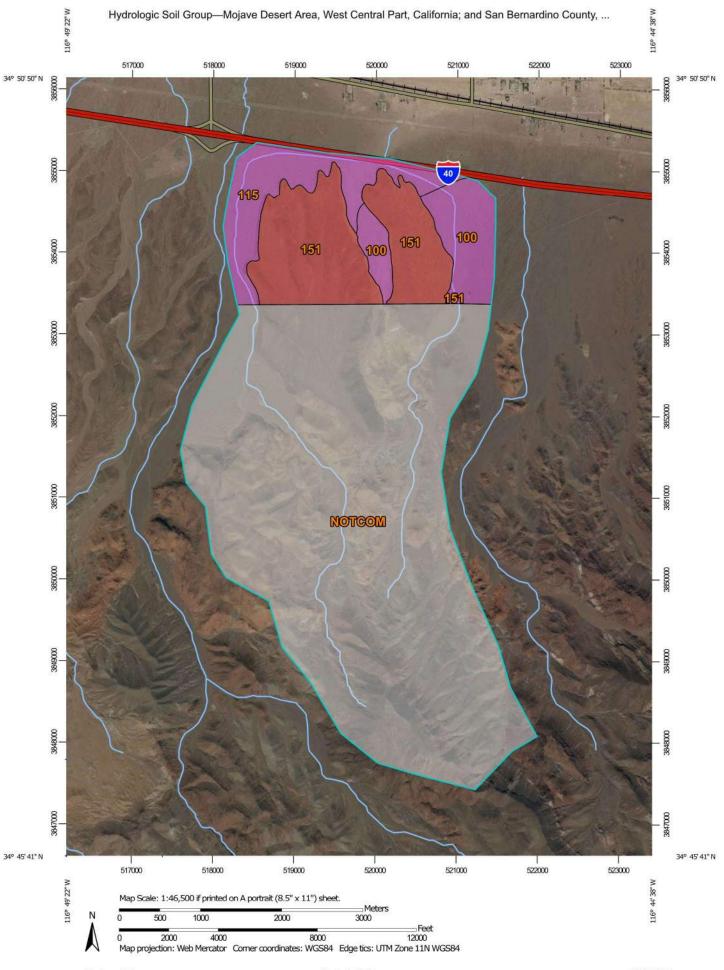
Rating Options

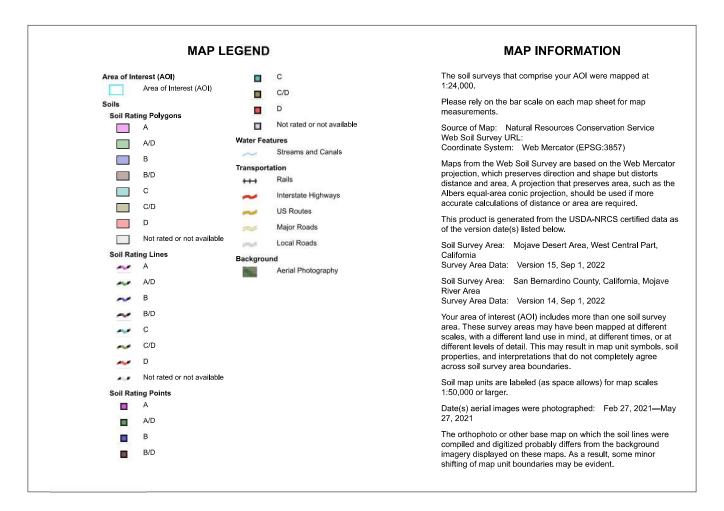
Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher







Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI				
NOTCOM	No Digital Data Available		4,068.9	73.9%				
Subtotals for Soil Survey Area			4,068.9					
Totals for Area of Intere	est	5,502.7	100.0%					

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
100	ARIZO GRAVELLY LOAMY SAND, 2 TO 9 PERCENT SLOPES	A	315.8	5.7%
115	CAJON GRAVELLY SAND, 2 TO 15 PERCENT SLOPES	A	390.8	7.1%
151	NEBONA-CUDDEBACK COMPLEX, 2 TO 9 PERCENT SLOPES*	D	727.1	13.2%
Subtotals for Soil Surv	rey Area		1,433.7	26.1%
Totals for Area of Inter	est		5,502.7	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOAA ATLAS 14 POINT RAINFALLS



NOAA Atlas 14, Volume 6, Version 2 Location name: Daggett, California, USA* Latitude: 34.8433°, Longitude: -116.7876° Elevation: 1942 ft**

* source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration				Averaç	ge recurrenc	e interval (y	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.099 (0.081-0.122)	0.138 (0.113-0.170)	0.194 (0.158-0.240)	0.243 (0.197-0.303)	0.316 (0.248-0.407)	0.378 (0.291-0.497)	0.446 (0.335-0.600)	0.522 (0.382-0.721)	0.635 (0.446-0.912)	0.730 (0.497-1.08)
10-min	0.142 (0.116-0.175)	0.198 (0.162-0.244)	0.277 (0.226-0.343)	0.348 (0.282-0.434)	0.454 (0.356-0.584)	0.542 (0.417-0.712)	0.640 (0.481-0.860)	0.748 (0.548-1.03)	0.910 (0.640-1.31)	1.05 (0.712-1.55)
15-min	0.172 (0.140-0.212)	0.239 (0.195-0.295)	0.335 (0.274-0.415)	0.421 (0.341-0.525)	0.548 (0.430-0.706)	0.656 (0.504-0.862)	0.774 (0.581-1.04)	0.905 (0.662-1.25)	1.10 (0.774-1.58)	1.27 (0.862-1.88)
30-min	0.230 (0.189-0.284)	0.321 (0.262-0.396)	0.450 (0.367-0.558)	0.565 (0.458-0.705)	0.736 (0.578-0.948)	0.881 (0.677-1.16)	1.04 (0.781-1.40)	1.22 (0.889-1.68)	1.48 (1.04-2.12)	1.70 (1.16-2.52)
60-min	0.312 (0.255-0.385)	0.434 (0.355-0.536)	0.609 (0.497-0.754)	0.765 (0.619-0.954)	0.996 (0.782-1.28)	1.19 (0.916-1.56)	1.41 (1.06-1.89)	1.64 (1.20-2.27)	2.00 (1.41-2.87)	2.30 (1.56-3.41)
2-hr	0.380 (0.311-0.469)	0.515 (0.421-0.636)	0.704 (0.575-0.872)	0.869 (0.704-1.08)	1.11 (0.871-1.43)	1.31 (1.01-1.72)	1.52 (1.15-2.05)	1.76 (1.29-2.43)	2.10 (1.48-3.02)	2.39 (1.63-3.55)
3-hr	0.440 (0.360-0.543)	0.591 (0.483-0.730)	0.801 (0.654-0.992)	0.983 (0.796-1.23)	1.24 (0.977-1.60)	1.46 (1.12-1.92)	1.69 (1.27-2.27)	1.94 (1.42-2.68)	2.30 (1.62-3.30)	2.59 (1.77-3.85)
6-hr	0.532 (0.436-0.656)	0.712 (0.582-0.879)	0.958 (0.781-1.19)	1.17 (0.945-1.46)	1.46 (1.15-1.89)	1.70 (1.31-2.24)	1.96 (1.47-2.63)	2.23 (1.63-3.08)	2.61 (1.84-3.75)	2.93 (1.99-4.34)
12-hr	0.587 (0.481-0.724)	0.787 (0.644-0.972)	1.06 (0.864-1.31)	1.29 (1.04-1.61)	1.61 (1.26-2.07)	1.86 (1.43-2.45)	2.13 (1.60-2.86)	2.41 (1.76-3.33)	2.81 (1.98-4.03)	3.13 (2.13-4.64)
24-hr	0.752 (0.667-0.865)	1.02 (0.902-1.17)	1.37 (1.21-1.59)	1.67 (1.46-1.94)	2.08 (1.76-2.50)	2.40 (2.00-2.96)	2.74 (2.22-3.45)	3.09 (2.44-4.00)	3.58 (2.71-4.83)	3.97 (2.90-5.54)
2-day	0.865 (0.768-0.995)	1.19 (1.05-1.37)	1.61 (1.42-1.86)	1.96 (1.72-2.28)	2.45 (2.07-2.94)	2.82 (2.34-3.47)	3.21 (2.60-4.04)	3.62 (2.85-4.68)	4.18 (3.16-5.63)	4.62 (3.37-6.44)
3-day	0.921 (0.817-1.06)	1.27 (1.13-1.47)	1.74 (1.54-2.01)	2.12 (1.86-2.47)	2.64 (2.24-3.18)	3.05 (2.53-3.75)	3.47 (2.81-4.37)	3.90 (3.08-5.05)	4.50 (3.40-6.07)	4.96 (3.63-6.93)
4-day	0.958 (0.850-1.10)	1.33 (1.18-1.53)	1.82 (1.61-2.10)	2.22 (1.95-2.59)	2.77 (2.35-3.34)	3.20 (2.66-3.93)	3.64 (2.95-4.58)	4.09 (3.22-5.30)	4.71 (3.56-6.36)	5.20 (3.80-7.26)
7-day	1.03 (0.912-1.18)	1.44 (1.28-1.66)	1.98 (1.75-2.29)	2.42 (2.12-2.82)	3.02 (2.56-3.64)	3.49 (2.90-4.28)	3.96 (3.21-4.98)	4.45 (3.51-5.76)	5.11 (3.87-6.90)	5.63 (4.12-7.86)
10-day	1.09 (0.963-1.25)	1.53 (1.36-1.76)	2.11 (1.86-2.43)	2.58 (2.26-3.00)	3.21 (2.72-3.87)	3.70 (3.08-4.55)	4.20 (3.40-5.29)	4.72 (3.72-6.10)	5.41 (4.09-7.30)	5.96 (4.35-8.32)
20-day	1.24 (1.10-1.43)	1.75 (1.55-2.02)	2.42 (2.13-2.79)	2.95 (2.59-3.44)	3.67 (3.11-4.42)	4.22 (3.51-5.19)	4.78 (3.88-6.02)	5.36 (4.22-6.93)	6.13 (4.64-8.28)	6.74 (4.93-9.41)
30-day	1.38 (1.23-1.59)	1.94 (1.72-2.24)	2.66 (2.36-3.08)	3.25 (2.85-3.78)	4.03 (3.42-4.85)	4.62 (3.84-5.68)	5.23 (4.24-6.58)	5.85 (4.61-7.57)	6.69 (5.06-9.02)	7.34 (5.37-10.3)
45-day	1.58 (1.41-1.82)	2.20 (1.95-2.53)	2.99 (2.64-3.46)	3.63 (3.18-4.22)	4.48 (3.80-5.39)	5.13 (4.26-6.30)	5.79 (4.69-7.29)	6.46 (5.10-8.37)	7.38 (5.58-9.95)	8.09 (5.91-11.3)
60-day	1.75 (1.56-2.02)	2.41 (2.13-2.77)	3.24 (2.86-3.74)	3.91 (3.43-4.56)	4.81 (4.08-5.79)	5.50 (4.56-6.75)	6.19 (5.01-7.79)	6.90 (5.44-8.93)	7.86 (5.94-10.6)	8.61 (6.29-12.0)

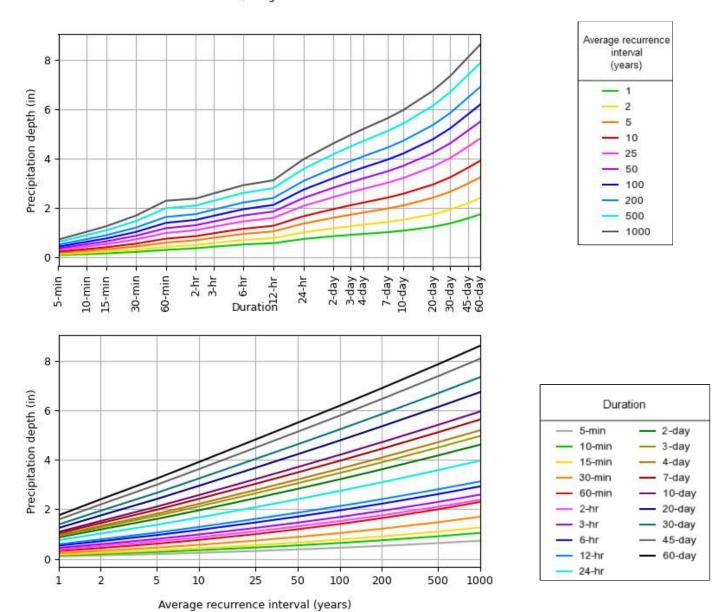
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PDS-based depth-duration-frequency (DDF) curves Latitude: 34.8433°, Longitude: -116.7876°



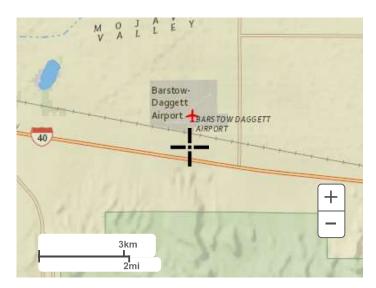
NOAA Atlas 14, Volume 6, Version 2

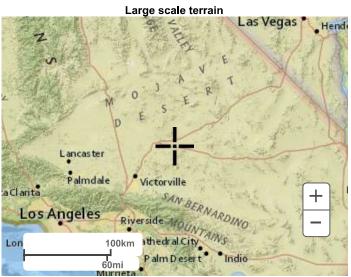
Created (GMT): Sun Jun 4 17:04:43 2023

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Maps & aerials

Small scale terrain







Large scale aerial



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US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC,Questions@noaa,gov

<u>Disclaimer</u>



NOAA Atlas 14, Volume 6, Version 2 Location name: Newberry Springs, California, USA*

Latitude: 34.8016°, Longitude: -116.7999° Elevation: 2975 ft**

* source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration				Averaç	ge recurrenc	e interval (y	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.096 (0.079-0.118)	0.136 (0.111-0.168)	0.192 (0.157-0.238)	0.241 (0.195-0.301)	0.314 (0.246-0.404)	0.374 (0.288-0.491)	0.440 (0.331-0.591)	0.512 (0.375-0.707)	0.619 (0.436-0.889)	0.709 (0.483-1.05)
10-min	0.138 (0.113-0.170)	0.195 (0.159-0.240)	0.275 (0.225-0.341)	0.346 (0.280-0.431)	0.450 (0.353-0.579)	0.536 (0.413-0.704)	0.630 (0.474-0.847)	0.734 (0.537-1.01)	0.887 (0.624-1.27)	1.02 (0.692-1.51)
15-min	0.166 (0.136-0.205)	0.235 (0.193-0.290)	0.333 (0.272-0.412)	0.418 (0.339-0.522)	0.544 (0.427-0.700)	0.649 (0.499-0.851)	0.762 (0.573-1.02)	0.888 (0.650-1.23)	1.07 (0.755-1.54)	1.23 (0.837-1.82)
30-min	0.224 (0.184-0.276)	0.317 (0.259-0.391)	0.448 (0.366-0.555)	0.563 (0.456-0.702)	0.732 (0.575-0.943)	0.873 (0.672-1.15)	1.03 (0.772-1.38)	1.20 (0.875-1.65)	1.44 (1.02-2.08)	1.66 (1.13-2.46)
60-min	0.306 (0.250-0.377)	0.432 (0.354-0.533)	0.611 (0.499-0.756)	0.768 (0.622-0.958)	0.998 (0.784-1.28)	1.19 (0.916-1.56)	1.40 (1.05-1.88)	1.63 (1.19-2.25)	1.97 (1.39-2.83)	2.26 (1.54-3.35)
2-hr	0.403 (0.330-0.497)	0.550 (0.450-0.679)	0.755 (0.616-0.934)	0.931 (0.754-1.16)	1.19 (0.932-1.53)	1.40 (1.08-1.83)	1.62 (1.22-2.18)	1.87 (1.37-2.58)	2.23 (1.57-3.20)	2.52 (1.72-3.74)
3-hr	0.480 (0.393-0.592)	0.648 (0.530-0.800)	0.880 (0.719-1.09)	1.08 (0.874-1.35)	1.36 (1.07-1.76)	1.60 (1.23-2.10)	1.84 (1.39-2.48)	2.11 (1.55-2.92)	2.50 (1.76-3.59)	2.82 (1.92-4.18)
6-hr	0.615 (0.504-0.759)	0.826 (0.676-1.02)	1.11 (0.908-1.38)	1.35 (1.10-1.69)	1.70 (1.33-2.18)	1.97 (1.52-2.59)	2.26 (1.70-3.04)	2.58 (1.88-3.56)	3.02 (2.12-4.34)	3.39 (2.31-5.03)
12-hr	0.746 (0.611-0.919)	0.999 (0.818-1.23)	1.34 (1.09-1.66)	1.63 (1.32-2.03)	2.03 (1.59-2.61)	2.35 (1.81-3.08)	2.68 (2.02-3.60)	3.04 (2.23-4.20)	3.56 (2.50-5.10)	3.98 (2.71-5.90)
24-hr	0.959 (0.851-1.10)	1.29 (1.14-1.48)	1.73 (1.53-2.00)	2.10 (1.84-2.44)	2.61 (2.22-3.14)	3.02 (2.51-3.71)	3.45 (2.79-4.34)	3.90 (3.07-5.05)	4.53 (3.43-6.12)	5.05 (3.69-7.05)
2-day	1.14 (1.01-1.31)	1.54 (1.36-1.77)	2.06 (1.82-2.38)	2.51 (2.20-2.92)	3.13 (2.66-3.77)	3.62 (3.01-4.45)	4.14 (3.35-5.21)	4.68 (3.69-6.06)	5.46 (4.13-7.36)	6.09 (4.45-8.50)
3-day	1.22 (1.08-1.41)	1.66 (1.47-1.91)	2.24 (1.98-2.58)	2.72 (2.38-3.16)	3.40 (2.88-4.09)	3.93 (3.27-4.83)	4.50 (3.65-5.66)	5.10 (4.02-6.60)	5.94 (4.50-8.02)	6.63 (4.84-9.26)
4-day	1.29 (1.14-1.48)	1.75 (1.55-2.02)	2.35 (2.08-2.72)	2.86 (2.51-3.33)	3.57 (3.03-4.30)	4.14 (3.43-5.08)	4.72 (3.83-5.95)	5.35 (4.22-6.93)	6.23 (4.71-8.41)	6.95 (5.08-9.71)
7-day	1.42 (1.26-1.63)	1.93 (1.71-2.22)	2.58 (2.28-2.98)	3.11 (2.73-3.62)	3.85 (3.27-4.64)	4.44 (3.69-5.46)	5.06 (4.10-6.37)	5.72 (4.51-7.40)	6.65 (5.03-8.97)	7.42 (5.42-10.4)
10-day	1.52 (1.35-1.74)	2.06 (1.83-2.37)	2.74 (2.42-3.17)	3.30 (2.89-3.84)	4.05 (3.44-4.88)	4.65 (3.86-5.71)	5.27 (4.27-6.64)	5.94 (4.68-7.69)	6.87 (5.20-9.27)	7.65 (5.59-10.7)
20-day	1.80 (1.60-2.08)	2.42 (2.14-2.79)	3.17 (2.80-3.66)	3.76 (3.30-4.38)	4.54 (3.85-5.46)	5.13 (4.26-6.30)	5.74 (4.65-7.23)	6.39 (5.04-8.28)	7.34 (5.55-9.90)	8.14 (5.94-11.4)
30-day	2.08 (1.85-2.40)	2.77 (2.46-3.19)	3.58 (3.16-4.13)	4.20 (3.68-4.88)	4.97 (4.22-5.98)	5.56 (4.62-6.84)	6.18 (5.01-7.78)	6.84 (5.39-8.86)	7.80 (5.90-10.5)	8.65 (6.32-12.1)
45-day	2.46 (2.18-2.83)	3.21 (2.85-3.70)	4.08 (3.60-4.71)	4.72 (4.13-5.49)	5.48 (4.64-6.59)	6.02 (5.00-7.40)	6.60 (5.35-8.31)	7.24 (5.70-9.37)	8.17 (6.18-11.0)	9.02 (6.59-12.6)
60-day	2.76 (2.45-3.17)	3.56 (3.15-4.10)	4.43 (3.92-5.12)	5.06 (4.44-5.90)	5.81 (4.93-6.99)	6.32 (5.25-7.76)	6.83 (5.53-8.60)	7.44 (5.86-9.63)	8.33 (6.30-11.2)	9.17 (6.70-12.8)

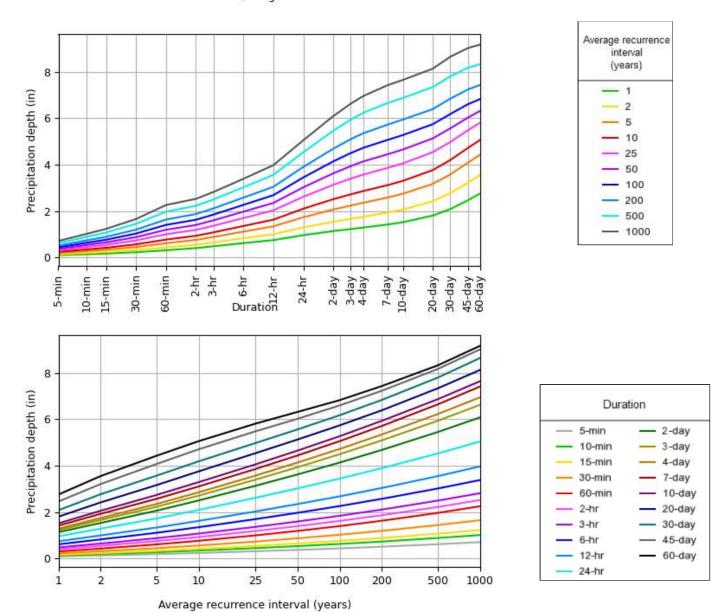
Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PDS-based depth-duration-frequency (DDF) curves Latitude: 34.8016°, Longitude: -116.7999°



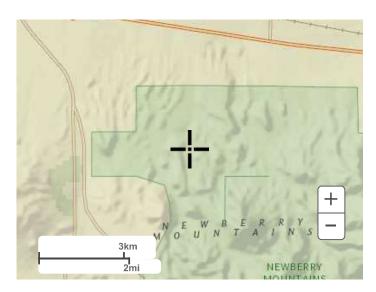
NOAA Atlas 14, Volume 6, Version 2

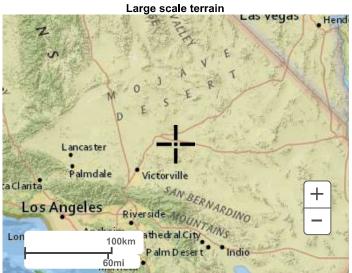
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