

GeoTek, Inc. 6835 South Escondido Street Suite A Las Vegas, Nevada 89119-3832 (702) 897-1424 Office (702) 897-2213 Fax www.geotekusa.com

> April 29, 2024 Project No. 17935-LV

ETT CA, LLC 5195 South Las Vegas Boulevard Las Vegas, Nevada 89119

Subject: On-Site Wastewater Disposal Design Recommendations Halloran Summit Travel Station Assessor's Parcel No. (APN) 0570061260000 Southeast Corner of I-15 and Halloran Summit Road San Bernardino County, California

As requested and authorized, GeoTek, Inc. (GeoTek) has prepared this report to present on-site wastewater disposal design recommendations for the proposed effluent disposal system that will service the new Travel Station that will be located on the southeast corner of I-I5 and Halloran Summit Road in San Bernardino County, California. This report presents the results of our evaluation, discussion of our findings and provides design and construction recommendations for the anticipated on-site wastewater disposal system. The disposal system will consist of a septic tank and leach fields.

The appropriate percolation test procedure was determined per the Local Agency Management Program (LAMP) and the Percolation Testing and Reporting Standards for Onsite Wastewater Treatment Systems by the San Bernadino County Public Health department. It is our understanding that no additional testing is being required by the County of San Bernardino for these design recommendations.

The opportunity to be of service is sincerely appreciated. If you should have any questions, please do not hesitate to call our office.

Respectfully submitted, **GeoTek, Inc.**

Colin J. O'Neill Staff Geologist



Ryne C. Stoker, P.E. Principal Engineer

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SCOPE OF SERVICES

The purpose of this study was to evaluate the percolation rates and physical characteristics of the onsite materials in order to provide design and construction recommendations for an on-site wastewater disposal system. Services provided for this study included the following:

- Review of available published and other data regarding geologic and soil conditions at the site, including previous subsurface exploratory work performed for the site by our firm.
- Compilation of this report that presents our findings and design recommendations for the proposed effluent disposal system.

SITE DESCRIPTION

The subject site consists of approximately 8.2 acres of partially developed land. The site is generally bound by the I-15 interstate to the north, the Mojave National Preserve to the east and south, and Halloran Summit Road to the west. The northern portion of the site consists of land developed with multiple abandoned structures with an active communication tower along the northern border. The southern portion of the site consists of disturbed land with various amounts of trash underlaying the surface soils. The site has slightly rolling hills to the south, but is otherwise relatively flat with surface drainage generally directed to the east. No rock outcroppings were observed within the limits of the site.

It is our understanding that the source of domestic water for the proposed development will be from a well that has yet to be sited and constructed. Three potential abandoned septic systems were observed across the site in preliminary investigations, but are not in the vicinity of the newly proposed effluent disposal system. The specific lateral extent of these abandoned fields are unknown but will need to be properly removed and the area graded.

GeoTek is not aware of any on-site features that may adversely affect the anticipated effluent disposal system but observation of the excavation for the proposed effluent disposal should be performed to identify any adverse soil conditions that may exist.

PROPOSED DEVELOPMENT

It is our understanding that site development would consist of the demolition of the existing abandoned structures and performing typical cut and fill earthwork to attain the desired graded configuration(s) for the construction of an approximately 7,500 square foot convenience store, parking log, fuel canopy, underground fuel tanks, retention basin and 9,000 gallon septic system with associated improvements for each. Finished grades in the area of the leach fields should be similar to existing grades (i.e. post-earthwork construction).

FIELD STUDIES

Subsurface conditions were explored using a truck mounted hollow stem auger drill rig. Twelve explorations were advanced onsite, in addition to seven percolation borings (P-I through P-6 and P-8). The diameter of the borings was approximately 8 inches. Logs of each exploration are included with this report in Appendix A. The locations and elevations reported on each log are estimates acquired from topographic mapping software and were not surveyed in the field. Field studies were completed in January 2024 by staff geologist, Colin J. O'Neill.

The appropriate percolation test procedure was determined per the Local Agency Management Program (LAMP) and the Percolation Testing and Reporting Standards for Onsite Wastewater Treatment Systems by the San Bernadino County Public Health department. A three-inch diameter perforated PVC pipe was placed in each of the percolation borings. Gravel approximately ³/₄-inch in size was place on the bottom 2 inches of the PVC pipe. Measurements, utilizing a measuring tape with I/8-inch subdivision, were taken at various rates for one to three hours. The rates of measurement as well as duration can be found on the percolation test sheets in Appendix B.

GEOLOGIC AND SOIL CONDITIONS

Subsurface Conditions

Based on our site reconnaissance, subsurface excavations, and review of published geologic maps, the area of the proposed on-site effluent disposal system is underlain by alluvium to the depths explored. In general, the alluvial materials typically consisted of slightly moist, medium to very dense sands with varying amounts of silt, clay and gravel and are considered to have "favorable" characteristics, in accordance with the current standards of the County of San Bernardino LAMP. A more detailed description of these materials is provided on the logs of exploratory borings in Appendix A.

Surface Water

Surface water was not observed during our site reconnaissance or investigation. If encountered during earthwork construction, surface water on this site is the result of precipitation or possibly some minor surface run-off from immediately surrounding areas. Overall site drainage is generally in an easternly direction, as directed by site topography. Provisions for surface drainage will need to be accounted for by the project civil engineer.

Groundwater

Groundwater was not encountered in any of the borings at the time of drilling. Based on a review of information contained on the California Department of Water Resources, Water Data Library, and the USGS Water Resources of the United States, groundwater is reported at a depth greater than 100 feet below ground surface. The depth to groundwater is expected to vary seasonally and localized perched groundwater conditions could be encountered. However, groundwater is not anticipated to impact the planned development.

PERCOLATION TESTING

GeoTek performed percolation testing on February 2, 2024 and February 3, 2024. Percolation testing was performed in general accordance with the *Percolation Testing and Reporting Standards for Onsite Wastewater Treatment Systems* of the San Bernardino County Public Health, Division of Environmental Health Services (2019).

Summary of Percolation Test Results

The percolation test data results are included in Appendix B. Based on the obtained rates, the on-site materials displayed adequate percolation rates for the design of an on-site wastewater disposal system in accordance with the current standards of the San Bernardino County Public Health, Division of Environmental Health Services (see reference).

Pre-Soaking

The borings (P-2, P-5, and P-8) were initially filled with clear water upon completion of excavation and were presoaked for at least 16 hours prior to commencement of testing. The remaining borings (P-1, P-3, P-4, P-6 and P-8) were presoaked with 12 inches (10 inches above 2 inches of gravel) of clear water that drained within 10 minutes. Percolation testing for these borings commenced immediately after the presoaking procedure.

Testing Procedure

All of the test holes remained open to the original drilled depth of approximately 3 feet due to the installation of the perforated PVC pipe in each of the holes. Measurements utilizing a measuring tape with 1/8-inch division, percolation rates were taken at various time intervals. Each measurement was taken at and the time interval noted and were representative of the percolation rate encountered. Copies of the data sheets are provided in Appendix B.

Percolation Results

Based on the results of the percolation testing, variability of percolation rates were encountered across the site. The percolations rates calculated for the site varied from between, approximately 2.11 minutes per inch to 160 minutes per inch.

In general, a majority of the results were typical of what would be expected for the geotechnical conditions encountered within the onsite explorations, with percolation rates ranging from 2.11 minutes per inch to 30 minutes per inch. However, one percolation rate was recorded with a rate of 160 minutes per inch (P-5). Based on the geotechnical conditions encountered, and the percolation rates of nearby tests, it is the opinion of the undersigned that this percolation rate (160 minutes per inch) is anomalous and should not be considered for septic design. Based upon the explorations and testing performed, it is recommended that a percolation rate of 30 minutes per inch should be utilized for design of the onsite septic system.

Excavation for the leach field should be observed by a representative of GeoTek to evaluate the condition of the soils encountered at the excavation limits, especially in the area of test P-5, to identify any adverse soil conditions that may impact soil percolation rates (i.e., impermeable soil layers, boulders, etc.). If any adverse soil conditions are encountered, specific earthwork recommendations can be provided to ensure that the conditions are suitable and in conformance with the septic system design.

DESIGN RECOMMENDATIONS

Based on conversations with the client, a 9,000-gallon septic tank is currently proposed for the onsite effluent disposal system. The following calculations are provided for the design of the leach field.

Most Conservative MPI Rate – 30.0 minute per inch* Application Rate (square foot per gallons per day) per the LAMP (2017) – 1.88 ft²/gal per day* Application Rate (1.88) x flow (3,500 gallons per day) = 6,580 ft² Absorption Area Absorption Area (6,580 ft²) / (9,000-gallon septic tank ÷ 100 gal) = 73.1 ft²/100 gstc Absorption Area / Trench Credit Area (7 ft²) = 940 Total Lineal Feet Design Leach Field = 24 Lines (40 feet long and 5 feet deep) Recommended Leach Field = 24 Lines (60 feet long and 5 feet deep) *In accordance with current standards of the County of San Bernardino LAMP

Based on the above preliminary data, it is our judgement that the following apply:

- The planting of trees and large shrubs should be avoided within the area of the septic tank and leach field.
- The planned on-site wastewater disposal system, if utilized and maintained properly, is not anticipated to adversely impact the site or adjacent properties.
- Based on the data presented in this report and using the recommendations set forth, it is the opinion of GeoTek that there is sufficient area on this site to support a primary and expansion on-site wastewater treatment system that will meet the current standards of the County of San Bernardino LAMP.
- The designed system shall be located within the area of the percolation tests performed onsite.
- Based on the data presented in this report and the testing information accumulated, it is the judgement of GeoTek that the groundwater table will not encroach within the current allowable limit set for the by the County of San Bernardino.

MINIMUM HORIZONTAL DISTANCE IN CLEAR REQUIRED FROM	TO LEACH FIELD		
Buildings or Structures*	8 Feet		
Property Line Adjoining Private Property	5 Feet		
Water Supply Wells	100 Feet		
Trees (greater than 10 inches in diameter)			
Seepage Pits	5 Feet		
Disposal Field	4 Feet**		
Onsite Domestic Water Service Line	5 Feet		
Disposal System Distribution Box	5 Feet		
Pressure Public Water Main			
Flood Plain/100 Year Flood Zone	Refer to Current Uniform Plumbing Code		
Groundwater	5 Feet		
Groundwater			

* Including porches and steps, whetere covered or uncovered, breezeways, roofed porte-cocheres, roofed patios, carports, covered walks, covered driveways and similar structures or appurtenances.

**Plus 2-feet for each additional foot or depth in excess of 1 foot below the bottom of the drain line.

INTENT

It is the intent of this report to aid in the design and construction of the proposed development. The professional opinions and geotechnical information contained in this report are not intended to imply total performance of the project or guarantee that unusual or variable conditions not be discovered during or after construction.

The scope of our study is limited to the area explored. This evaluation does not and should in no way be construed to encompass any areas beyond the specific area of the proposed construction as indicated to us by the client. The scope is based on our understanding of the project and the client's needs and geotechnical engineering standards normally used on similar projects in this region.

The materials observed on the project site appear to be representative of the area; however, soil conditions and natural materials vary in character between excavations and natural outcrops or conditions exposed during site construction. Site conditions may vary due to seasonal changes or other factors. GeoTek, Inc. assumes no responsibility or liability for work, testing or recommendations performed or provided by others.

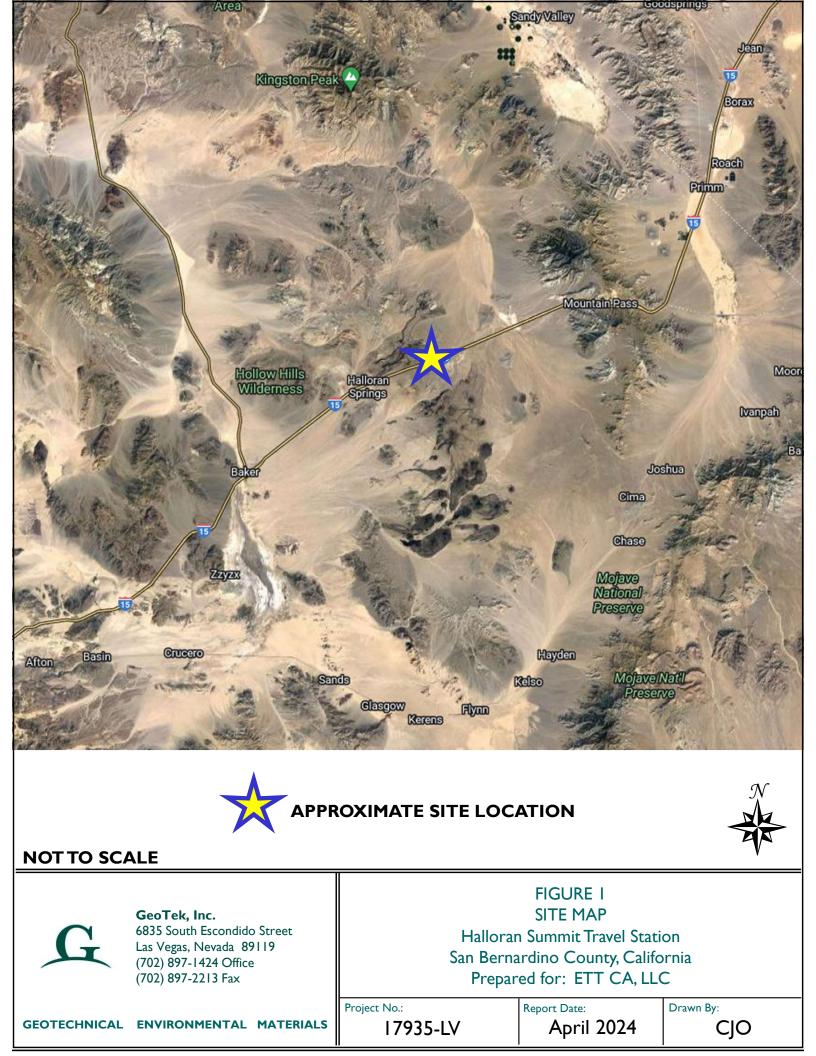
Our recommendations are based on the site conditions observed and encountered, and laboratory testing. Our conclusions and recommendations are professional opinions that are limited to the extent of the available data. Observations during construction are important to allow for any change in recommendations found to be warranted. These opinions have been derived in accordance with current standards of practice and no warranty is expressed or implied. Standards of practice are subject to change with time.

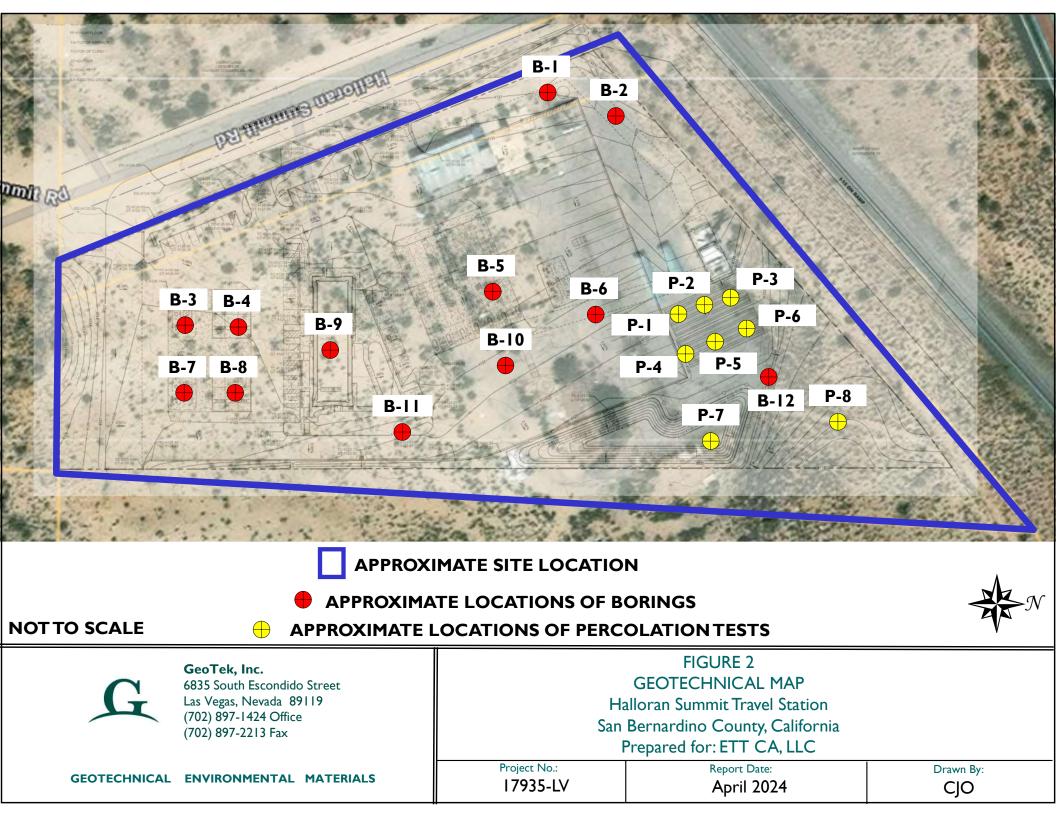
REFERENCES

San Bernardino County Public Health, Division of Environmental Health Services, 2017, "Local Agency Management Program (LAMP) for Onsite Wastewater Treatment Systems," dated May.

San Bernardino County Public Health, Division of Environmental Health Services, 2019, "Percolation Testing and Reporting Standards for Onsite Waste Water Treatment Systems," dated September.







APPENDIX A

BORING LOG GENERAL NOTES

CONSISTENCY OF FINE-GRAINED SOILS

Unconfined Compressive Strength, Qu, psf	Standard Penetration or N- Value (SS) Blows/Ft	Consistency
< 500	<2	Very Soft
500 - 1,000	2 - 3	Soft
1,001 - 2,000	4 - 7	Firm
2,001 - 4,000	8 - 16	Stiff
4,001 - 8,000	17 - 32	Very Stiff
> 8,001	32+	Hard

RELATIVE DENSITY OF COARSE-GRAINED SOILS
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Standard Penetration (SPT) or N-Value (SS) Blows/Ft	Relative Density	
0 - 3	Very Loose	
4 - 9	Loose	
10 - 29	Medium Dense	
30 - 49	Dense	
50+	Very Dense	

SPT penetration test using 140 pound hammer, with 30 inch free fall on 2 inch outside diameter(1-3/8 ID) sampler For ring sampler using 140 lb hammer, with a 30 inch free fall on 3 inch outside diameter (2-1/2 ID) sample, use N-value x 0.636 to get Standard N-value

For fine grained soil consistency, thumb penetration used per ASTM D-2488

RELATIVE PROPORTIONS OF SAND AND GRAVEL							
Descriptive Term of other Constituents Constituents Constituents							
Trace	< 15						
With	15 - 29						
Modifier	> 30						

GRAIN SIZE TERMINOLOGY						
Major Component of Particle Size Sample						
Boulders	Over 12 inches					
Cobbles	3 inches to 12 inches					
Gravel	#4 Sieve to 3 inches					
Sand	#200 Sieve to #4 Sieve					
Silt or Clay	Passing #200 Sieve					

RELATIVE HARDNESS OF CEMENTED SOILS (CALICHE)							
Description General Characteristics							
Very Dense to Moderately Hard	Partially Cemented Granular Soil - Can be carved with a knife and broken with force by hand.						
Very Stiff to Moderately Hard	Partially Cemented Fine-Grained Soil - Can be carved with a knife and broken with force by hand.						
Moderately Hard	derately Hard Moderate hammer blow required to break a sample						
Hard	Heavy hammer blow required to break a sample						
Very Hard Repeated heavy hammer blow required to break a sample							

MOISTURE CLASSIFICATION						
Description*	Degree of Saturation					
Dry	0%					
Slightly Moist	1% - 50%					
Moist	51%-75%					
Wet	76% - 99%					
Saturated	Saturated 100%					
*Defined as Condition of Sand						

BORING LOG LEGEND

	MATERIAL DESCRIPTION							
Soil Pattern	USCS Symbol	USCS Classification						
	FILL	Artificial Fill						
	GP or GW	Poorly/Well graded GRAVEL						
	GM	Silty GRAVEL						
	GC	Clayey GRAVEL						
	GP-GM or GW-GM	, ,						
	GP-GC or GW-GC	Poorly/Well graded GRAVEL with Clay						
	GC-GM	Silty Clayey GRAVEL						
	SP or SW	Poorly/Well graded SAND						
	SM	Silty SAND						
	SC	Clayey SAND						
	SP-SM or SW-SM	Poorly/Well graded SAND with Silt						
	SP-SC or SW-SC	Poorly/Well graded SAND with Clay						
	SC-SM	Silty Clayey SAND						
	ML	SILT						
	MH	Elastic SILT						
	CL-ML	Silty CLAY						
	CL							
	CH							
	PCEM CEM	PARTIALLY CEMENTED						
	•=							
	BDR	BEDROCK						

SAMPLING							
SPT							
	Ring Sample						
NR	No Recovery						
\geq	Bulk Sample						
\leq	Water Table						

	CONSISTENCY						
Co	Cohesionless Soils Cohesive Soils Cementation Bedrock					Bedrock	
VL	Very Loose	So	Soft	MH	Mod. Hard	ESt	Extremely Strong
L	Loose	F	Firm	н	Hard	VSt	Very Strong
MD	Medium Dense	S	Stiff	VH	Very Hard	St	Strong
D	Dense	VS	Very Stiff			MSt	Moderately Strong
VD	Very Dense					W	Weak
						Fr	Friable



PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 36.4012°, -115.7904°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/19/24
ELEVATION:	≈ 4136 ft

	SA	AMPL	ES	l			LABO	ORATO		STING
Depth (ft)	Sample Type	Blows/6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-1 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
	•1			FILL	(SM) Light Brown Silty SAND with Gravel; Slightly Moist	L	2		щ	<u> </u>
	/					MD				
1 -	\mathbb{V}			SM	Light Brown Silty SAND with Gravel; Slightly Moist	MD				
2 -	Å	27								
3 - 4 -	/	35 50/5"		SC	Light Brown Clayey SAND with Gravel; Slightly Moist	MD	2.9 2.3	117.8 120.1		
5 -		8		SM	Light Brown Silty SAND with Gravel, trace Clay; Slightly	D				
6 -		19 18			Moist					
7 -				SC	Light Brown Clayey SAND; Slightly Moist	D				
8 -										
9 -										
10 —		22		PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel; Slightly Moist	MH				
11 🗕		50/2"			- Drill Rate: 30 sec/ft @ 1800 psi					
12 -										
13 🗕					- Drill Rate: 30 sec/ft @ 1000 psi					
14 —					- Color Change: Greenish Brown					
15 -		45								
16 —		50/1"			Boring Ends at Approximately 16.0 Feet Depth					
17 🗕					No Groundwater Encountered					
18 –										
19 -										
20 -										



LOCATION:	≈ 35.4014°, -115.7903°
CLIENT:	ETT CA, LLC
PROJECT:	Halloran Summit Travel Center
PROJECT #:	17935-LV

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/19/24
ELEVATION:	≈ 4138 ft

	SA	MPL	ES	1			LABO	ORATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-2 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
<u> </u>				FILL	(SC) Light Brown Clayey SAND with Gravel; Slightly Moist	L	~		Н	<u> </u>
	$\backslash /$				(GC) Light Drown Clayey SAND with Cravel, Slightly Moist	MD				
1 -	Х			SC	Light Brown Clayey SAND with Gravel; Slightly Moist	MD				
2 -	/ \									
2 -						D				
3 -		17		SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel;	VD				
-		50/5"			Slightly Moist		1.8	124.5		
4 -										
5 -		21								
•		50/5"								
6 -										
7 -										
'				SC	Light Brown Clayey SAND with Gravel; Slightly Moist	MD				
8 -										
-										
9 -				PCEM	Light Greenish Brown PARTIALLY CEMENTED Sand and	MH				
				PCEIN	Gravel; Slightly Moist					
10 -		23			- Drill Rate: 30 sec/ft @ 1800 psi					
		50/5"								
11 🗕										
12 -										
12										
13 -										
14 -					- Drill Rate: 45 sec/ft @ 1800 psi					
15 🗕		22								
10		50/3"								
16 -					Boring Ends at Approximately 16.0 Feet Depth	1				
17 🗕					No Groundwater Encountered					
·′ -										
18 -										
19 🗕										
20 -										

G	
G E O T E K	

LOCATION:	≈ 35.4001°, -115.7893°
CLIENT:	ETT CA, LLC
PROJECT:	Halloran Summit Travel Center
PROJECT #:	17935-LV

	LOGGED BY:	CJO
	METHOD:	H. S. A.
r	DRILLER:	BC2
	DATE:	1/18/24
	ELEVATION:	≈ 4131 ft

	SA	MPL	ES	I			LABO	ORATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-3 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL	(SM) Light Brown Silty SAND with Gravel; Slightly Moist		V		щ	0
1 - 2 -	X			FILL		MD				
		7			- No Gravel					
3 -		8 8		SM	Light Brown Silty SAND, trace Gravel; Slightly Moist	MD				
					- Thin Partially Cemented Layer	D				
5 -		17				VD				
6 -		39 50/3"								
7 -				SC-SM	Light Brown Silty, Clayey SAND with Gravel, trace Gypsum;	VD				
8 -					Slightly Moist					
9 -				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel; Slightly Moist	MH				
10 —		24			- Drill Rate: 45 sec/ft @ 1800 psi					
11 -		44 50/3"								
12 -										
13 🗕										
14 —										
15 🗕		34								
16 –		50/2"			Boring Ends at Approximately 16.0 Feet Depth					
17 🗕					No Groundwater Encountered					
18 -										
19 🗕										
20 -										
L						1				

G	
GEOTEK	

 PROJECT #:
 17935-LV

 PROJECT:
 Halloran Summit Travel Center

 CLIENT:
 ETT CA, LLC

 LOCATION:
 ≈ 35.4002°, -115.7894°

 LOGGED BY:
 CJO

 METHOD:
 H. S. A.

 DRILLER:
 BC2

 DATE:
 1/18/24

 ELEVATION:
 ≈ 4131 ft

	SA	MPL	ES	Ţ				ORATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-4 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
-	•1			FILL	(SM) Light Brown Silty SAND; Slightly Moist	L	2		ш 0	<u> </u>
1 -						MD			0	
3 =		33 50/5"		SM	Light Brown Silty SAND; Slightly Moist	D VD				
5 -		10								
6 -		33 50/3"					1.7	127.5		
7 - 8 -				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel; Slightly Moist	МН				
9 -					- Drill Rate: 30 sec/ft @ 1500 psi					
10 -		32 50/4"			- Thin Non-Cemented Layer					
11 - 12 -										
13 -					- Drill Rate: 30 sec/ft @ 1200 psi					
14 - 15 -										
16 -		36 50/3"			Boring Ends at Approximately 16.0 Feet Depth					
17 -					No Groundwater Encountered					
18 - 19 -										
20 –										



PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4011°, -115.7896°

	LOGGED BY:	CJO
	METHOD:	H. S. A.
r	DRILLER:	BC2
	DATE:	1/18/24
	ELEVATION:	≈ 4134 ft

	SA	AMPL	ES	10				ORATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-5 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
-				FILL	(SC) Light Brown Clayey SAND with Gravel; Slightly Moist	L	-			
1 -	\bigvee				- Metal Pipe	MD				
2 -	Ň					D				
3 -	/	16		SC-SM	Light Brown Silty, Clayey SAND with Gravel; Slightly Moist	VD				
4 -	\square	49 50/4"			- With Gypsum					
5 -										
6 -		16 24 20		SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel; Slightly Moist	D				
7 -										
				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel;	MH				
8 -					Slightly Moist					
9 -					- Drill Rate: 30 sec/ft @ 1000 psi					
Ũ										
10 -		44								
		50/3"								
11 -					- Drill Rate: 30 sec/ft @ 1500 psi					
12 -										
					- Thin Cemented Layer					
13 🗕					- Thin Cemented Layer					
14 -					,					
14										
15 🗕		50/5"								
10		50/5			Boring Ends at Approximately 15.5 Feet Depth					
16 -					No Groundwater Encountered					
17 -										
18 -										
19 -										
20 -										



PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4014°, -115.7894°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/18/24
ELEVATION:	≈ 4132 ft

	SAMPLES _					LABO	DRATO	RY TE	STING	
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-6	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
	S	н	•1		MATERIAL DESCRIPTION AND COMMENTS		M	Ι	Щ	0
					2" AC Pavement over SC-SM; Slightly Moist	MH				
1 -	Х			FILL	(SC-SM) Light Brown Silty, Clayey SAND; Slightly Moist	MD				
2 -				SM	Light Brown Silty SAND, trace Gravel; Slightly Moist	D				
		10		_		VD				
3 -		31								
4		50					2.9	120.9		
4 -										
5 -										
5		19								
6 -		18								
Ŭ		17								
7 -										
-				SC	Light Brown Clayey SAND with Gravel; Slightly Moist	D				
8 -										
				DOEM	Light Brown PARTIALLY CEMENTED Sand and Gravel;	MH				
9 -				FCEIVI						
					Slightly Moist - Drill Rate: 15 sec/ft @ 1000 psi					
10 -		9								
		9 37								
11 -		46								
		40			- Drill Rate: 30 sec/ft @ 1200 psi					
12 -										
13 -										
14 -										
15 -										
15		27								
16 -		50/4"								
10					Boring Ends at Approximately 16.0 Feet Depth					
17 -					No Groundwater Encountered					
1										
18 -										
19 -										
20 -										
I					L					



PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4001°, -115.7890°

	LOGGED BY:	CJO
	METHOD:	H. S. A.
er	DRILLER:	BC2
	DATE:	1/18/24
	ELEVATION:	≈ 4128 ft

	SA	AMPL	ES	1				ORATO		STING
Depth (ft)	Sample Type	Blows/6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-7 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL	(SC-SM) Light Brown Silty, Clayey SAND; Slightly Moist	L				
1 - 2 -	Х					MD D				
		49		SM	Light Brown Silty SAND with Gravel; Slightly Moist	VD				
3 -		46 50/3"					2.5	123.9		
5 -				SC	Light Brown Clayey SAND, trace Gravel; Slightly Moist	VD				
5	NR	50/4"								
6 -										
7 -				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel;	MH				
8 -					Slightly Moist					
9 -					- Drill Rate: 30 sec/ft @ 1500 psi					
9										
10 -	NR	50/5"								
11 -										
12 -										
13 -										
14 -										
15 -										
		50/3"			Boring Ends at Approximately 15.5 Feet Depth					
16 —					No Groundwater Encountered					
17 🗕										
18 —										
19 🗕										
20 —										



 PROJECT #:
 17935-LV

 PROJECT:
 Halloran Summit Travel Center

 CLIENT:
 ETT CA, LLC

 LOCATION:
 ≈ 35.4003°, -115.7890°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/18/24
ELEVATION:	≈ 4128 ft

	SA	MPL	ES	10				ORATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-8 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL	(GM) Light Brown Silty GRAVEL with Sand, trace Organics;	L				-
1 🗕	Х				Slightly Moist	MD				
2 -				FILL	(SC-SM) Light Brown Silty, Clayey SAND; Slightly Moist	MD				
3 -		7								
4 -		13 11					2.7	125.8		
5 -						D				
0		15 22			- Trash in Sampler					
6 -		50/4"		SM	Light Brown Silty SAND, trace Gravel; Slightly Moist	VD	3.6	115.6		
7 -										
8 -										
9 -				SC	Light Brown Clayey SAND; Slightly Moist	VD				
10 -		33								
11 -		50/2"		PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel; - Drill Rate: 30 sec/ft @ 1500 psi	MH	4.1	120.1		
12 -										
13 🗕					- Drill Rate: 15 sec/ft @ 1000 psi					
14 -										
15 -		34								
16 –		44 50/2"								
17 -					Boring Ends at Approximately 16.5 Feet Depth No Groundwater Encountered					
18 –										
19 🗕										
20 –										
	I									

G	
GЕОТЕК	

 PROJECT #:
 17935-LV

 PROJECT:
 Halloran Summit Travel Center

 CLIENT:
 ETT CA, LLC

 LOCATION:
 ≈ 35.4005°, -115.7892°

LOGGED BY: CJO METHOD: H. S. A. DRILLER: BC2 DATE: 1/18/24 ELEVATION: ≈ 4130 ft

	S/	MPL	ES	I			LABO	ORATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-9 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL	(SC) Light Brown Clayey SAND; Slightly Moist	L	-		_	-
1 -						MD				
-	X	7		SC	Light Brown Clayey SAND; Slightly Moist - Increase in Sand	D				
3 - 4 -		7 24 50/4"		SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel; Slightly Moist	VD	3.1	114.9		
5 -	/ \ NR	36 50/2"								
6 - 7 -										
8 - 9 -				SM	Light Yellowish Brown Silty SAND; Slightly Moist	VD				
				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel;	MH				
10 - 11 -		35 50/3"		I OLM	Slightly Moist - Drill Rate: 30 sec/ft @ 1200 psi					
12 –					- Drill Rate: 30 sec/ft @ 1800 psi					
13 - 14 -										
15 -		22								
16 -		50/4"			Poring Endo at Annrovimatoly 16 0 East Darth	┨				
17 🗕					Boring Ends at Approximately 16.0 Feet Depth No Groundwater Encountered					
18 - 19 -										
20 –										



PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4011°, -115.7892°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/18/24
ELEVATION:	≈ 4131 ft

	SA	MPL	ES	1			LABO	ORATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-10 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL		MH	r			
1 🗕	V			FILL	2" AC Pavement over SC; Slightly Moist (SC) Light Brown Clayey SAND; Slightly Moist	MD				
2 -	$ \Lambda $			SC	Light Brown Clayey SAND, trace Gypsum; Slightly Moist	D				
3 -	/	7				VD				
0		22								
4 —	NR	35								
5 -		17		SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel;	D				
6		22		0, 00	Slightly Moist					
6 -		20								
7 -										
8 -				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel;	MH				
					Slightly Moist - Drill Rate: 15 sec/ft @ 1200 psi					
9 -										
10 -										
		44 50/3"								
11 🗕		50/5								
12 -										
12 -										
13 🗕					- Drill Rate: 30 sec/ft @ 1200 psi					
14 -					- Color Change: Light Yellowish Brown					
15 -		50/5"								
4.6		50/5			Boring Ends at Approximately 15.5 Feet Depth	+				
16 -					No Groundwater Encountered					
17 🗕										
18 –										
19 -										
20 -										

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G E O T E K	

PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4008°, -115.7890°

OGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/18/24
ELEVATION:	≈ 4127 ft

	SA	MPL	ES	7			LABO	DRATO		STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-11 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL	(SC) Light Brown Clayey SAND, trace Gravel; Slightly Moist		2		щ	<u> </u>
1 - 2 -	X					MD				
2 -		40		FILL	(SM) Light Brown Silty SAND; Slightly Moist	D				
3 - 4 -		10 17 21			- Trash in Sampler		3.1	111.8		
5 -										
6 -		23 50/4"		SC	Light Brown Clayey SAND; Slightly Moist	VD				
7 -				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel;	MH				
, 8 —					Slightly Moist - Drill Rate: 30 sec/ft @ 1200 psi					
9 - 10 -		42 50/4"								
12 - 13 - 14 -					- Drill Rate: 45 sec/ft @ 1500 psi					
15 -										
		50/5"			Boring Ends at Approximately 15.5 Feet Depth					
16 –					No Groundwater Encountered					
17 -										
18 –										
19 -										
20 -										

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PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4019°, -115.7892°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	1/19/24
ELEVATION:	≈ 4127 ft

	SAMPLES				LABORATORY TES			STING		
Depth (ft)	Sample Type	Blows/6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: B-12 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Expansion Index	Other Testing
				FILL	2" AC Pavement over GM; Slightly Moist	MH	F			
1 -	V			FILL	(GM) Dark Brown Silty GRAVEL with Sand; Slightly Moist	MD				
2 -	/	17			- Color Change: Brown	D VD				
4 -		36 43			Light Brown Poorly Graded SAND with Clay and Gravel; Slightly Moist	VD	3.6 4.0	125.4 125.4		
5 - 6 -		18 24 27		SP	Light Brown Poorly Graded SAND; Slightly Moist	VD				
7 - 8 -				SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel;	VD				
9 - 10 -	\bigvee			01-00	Slightly Moist - Decrease in Sand	VD				
11 -	$/ \setminus$	28 50/5"			- Increase in Clay					
12 -				PCEM	Light Brown PARTIALLY CEMENTED Sand and Gravel; Slightly Moist - Drill Rate: 15 sec/ft @ 1000 psi	MH				
13 🗕										
14 —										
15 -		17 50/5"								
17 -					Boring Ends at Approximately 16.0 Feet Depth No Groundwater Encountered					
18 –										
19 -										
20 –										



PROJECT #:	17935-LV		
PROJECT:	Halloran Summit Travel Center		
CLIENT:	ETT CA, LLC		
LOCATION:	≈ 35.4018°, -115.7895°		

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4127 ft

	SA	MPL	ES	I			LABO	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-1 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Swell (%)	Other Testing
				FILL	(SM) Dark Brown Silty SAND with Gravel; Slightly Moist	L				
1 🗖						MD				
2 -					Light Brown Poorly Graded SAND with Clay and Gravel; Slightly Moist	D				
3 -					Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered					
5 -										
6 -										
7 -										
8 - 9 -										
9 – 10 –										
11 -										
12 –										
13 🗕										
14 -										
15 -										
16 - 17 -										
17										
19 -										
20 –										

G	
GЕОТЕК	

PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4017°, -115.7895°

CJO
H. S. A.
BC2
2/22/24
≈ 4127 ft

	SA	MPL	ES	П			LABO	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-2	Consistency	Water Content (%)	Dry Density (pcf)	Swell (%)	Other Testing
	S	Ц	~ 1		MATERIAL DESCRIPTION AND COMMENTS	-	И	Ι		0
1 - 2 -				FILL	(SC) Dark Brown Clayey SAND with Gravel; Slightly Moist	L MD				
3 - 4 -					Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered					
5 -										
6 - 7 -										
8 -										
9 -										
10 -										
11 🗕										
12 -										
13 -										
14 -										
15 -										
16 -										
17 - 18 -										
19 -										
20 -										



PROJECT #:	17935-LV		
PROJECT:	Halloran Summit Travel Center		
CLIENT:	ETT CA, LLC		
LOCATION:	≈ 35.4017°, -115.7894°		

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4127 ft

Indext of the Los Indext of the Los </th <th>ing</th>	ing
1 MD 2 SP-SC 3 Sightly Moist 3 Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered 5 Image: State of the	Other Testing
1 SP-SC Light Brown Poorly Graded SAND with Clay and Gravel; MD 2 Slightly Moist MD 3 Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered Image: Comparison of the sector o	
2 - SP-SC Light Brown Poorly Graded SAND with Clay and Gravel; MD 3 - Boring Ends at Approximately 3.0 Feet Depth 4 - 5 - I I I I I I I I I I I I I I I I I	
3 Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered 5 Image: Constraint of the set o	
3 Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered 5 Image: Constraint of the sector of the	
4 - No Groundwater Encountered	
4 - No Groundwater Encountered 5 - Image: State of the state of	
4 - 5 -	
8 –	
9 -	
10 -	
12 -	
13 -	
15 -	
16 -	
17 -	
19 -	
20 -	



PROJECT #:	17935-LV		
PROJECT:	Halloran Summit Travel Center		
CLIENT:	ETT CA, LLC		
LOCATION:	≈ 35.4018°, -115.7894°		

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4127 ft

	SA	MPL	ES	F			LABO	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-4 MATERIAL DESCRIPTION AND COMMENTS	Consistency		Dry Density (pcf)	Swell (%)	Other Testing
				FILL	(SM) Dark Brown Silty SAND with Gravel; Slightly Moist	L				
1 -						MD				
2 -				SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel; Slightly Moist	MD				
3 🗕					Boring Ends at Approximately 3.0 Feet Depth					
4 -					No Groundwater Encountered					
5 -										
6 - 7 -										
8										
9 -										
10 -										
11 🗕										
12 –										
13 🗕										
14 -										
15 -										
17										
18 -										
19 -										
20 –										

G	
G E O T E K	

CLIENT: ETT CA, LLC	PROJECT: Halloran Summit Travel Center
LOCATION: ≈ 35.4018°, -115.7894°	
	 CLIENT: ETT CA, LLC

OGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4127 ft

	SA	MPL	ES	I			LABC	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-5 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Swell (%)	Other Testing
				FILL	(SC) Dark Brown Clayey SAND with Gravel; Slightly Moist	L	-			
1 - 2 -					(SC) Dark blown Clayey SAND with Cravel, Signify Wolst	MD				
3 - 4 -					Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered					
5 -										
6 -										
7 -										
8 -										
9 -										
10 -										
12 –										
13 🗕										
14 –										
15 🗕										
16 -										
17 –										
18 - 19 -										
20 -										



PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4017°, -115.7894°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4127 ft

	SA	MPL	ES	1			LABC	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows/6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-6 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Swell (%)	Other Testing
				FILL	(SM) Dark Brown Silty SAND with Gravel; Slightly Moist	L				
1 🗕						MD				
. –				SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel;	MD				
2 -					Slightly Moist					
3 🗕		-			Boring Ends at Approximately 3.0 Feet Depth					
					No Groundwater Encountered					
4 -										
5 -										
5										
6 -										
7 -										
•										
8 -										
9 -										
0										
10 -										
11 🗕										
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12 -										
13 🗕										
14 🗕										
15 🗕										
16 -										
10										
17 🗕										
18 🗕										
10										
19 🗕										
20 -										
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PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4018°, -115.7890°

LOGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4127 ft

	SA	MPL	ES				LABO	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-6 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Swell (%)	Other Testing
				FILL	(SM) Dark Brown Silty SAND with Gravel; Slightly Moist	L				
1 -						MD				
2 -										
3 - 4 -				SP-SC	Light Brown Poorly Graded SAND with Clay and Gravel; Slightly Moist	MD				
5 -										
6 -										
7 - 8 -										
9 -					Boring Ends at Approximately 8.0 Feet Depth No Groundwater Encountered					
10 –										
11 -										
12 - 13 -										
14 –										
15 🗕										
16 -										
17 - 18 -										
19 -										
20 –										

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PROJECT #:	17935-LV
PROJECT:	Halloran Summit Travel Center
CLIENT:	ETT CA, LLC
LOCATION:	≈ 35.4021°, -115.7891°

OGGED BY:	CJO
METHOD:	H. S. A.
DRILLER:	BC2
DATE:	2/22/24
ELEVATION:	≈ 4120 ft

	SA	MPL	ES	Ic				RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: P-8 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Dry Density (pcf)	Swell (%)	Other Testing
				SC	Light Brown Clayey SAND with Gravel; Slightly Moist	L	~			-
1 -						MD				
2 -						D				
3 -										
4 -					Boring Ends at Approximately 3.0 Feet Depth No Groundwater Encountered					
5 🗕										
6 -										
7 -										
8 -										
9 -										
10 - 11 -										
12 -										
13 🗕										
14 -										
15 🗕										
16 –										
17 🗕										
18 –										
19 🗕										
20 -										

APPENDIX B

TEST NUMBER P-1

PROJECT NAME	Halloran Summit Travel Stat	ion DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road		
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	N/A - Fast Perc	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3		

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)		(2)	(1) / (2)	
1:40	-	24	-	-	Start
1:47	7	34	10	0.70	Test
1:47	-	24	-	-	Refill
1:57	10	34	10	1.00	Test
		Water Drained	d within 10 Minut	es - Start Percolation Test	
1:57	-	28	-	-	Refill
2:07	10	31 3/4	3 3/4	2.67	Test
2:07	-	28	-	-	Refill
2:17	10	30 1/2	2 1/2	4.00	Test
2:17	-	28	-	-	Refill
2:27	10	30	2	5.00	Test
2:27	-	28	-	-	Refill
2:37	10	29 1/4	1 1/4	8.00	Test
2:37	-	28	-	-	Refill
2:47	10	29 1/4	1 1/4	8.00	Test
2:47	-	28	-	-	Refill
2:57	10	29 1/4	1 1/4	8.00	Test/Finish

SOIL DATA

0.0-1.5' FILL (SM) Dark Brown Silty Sand with Gravel; Slightly Moist

1.5-3.0' (SP-SC) Light Brown Poorly Graded Sand with Clay and Gravel; Slightly Moist

TEST NUMBER P-2

PROJECT NAME	Halloran Summit Travel Static	n DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road	_	
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	20	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3	-	

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)		(2)	(1) / (2)	
1:04	-	24	-	-	Start
1:14	10	28	6	1.67	Test
1:14	-	24	-	-	Refill
1:24	10	26	4	2.50	Test
Water	Did Not Drain C	ut in 10 Minute	s - Presoak Proc	eedure - Maintained 10" Head	for at least 4 Hours
9:40	-	28	-	-	Start
10:10	30	29 1/4	1 1/4	24.00	Test
10:10	-	28	-	-	Refill
10:40	30	28 7/8	7/8	34.29	Test
10:40	-	28	-	-	Refill
11:10	30	29	1	30.00	Test
11:10	-	28	-	-	Refill
11:40	30	29	1	30.00	Test
11:40	-	28	-	-	Refill
12:10	30	29	1	30.00	Test
12:10	-	28	-	-	Refill
12:40	30	29	1	30.00	Test/Finish

SOIL DATA

0.0-3.0' FILL (SC) Light Brown Clayey Sand with Gravel; Slightly Moist

TEST NUMBER P-3

PROJECT NAME	Halloran Summit Travel Statio	on DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road		
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	N/A - Fast Perc	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3		

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)		(2)	(1) / (2)	
11:19	-	24	-	-	Start
11:29	10	34	10	1.00	Test
11:29	-	24	-	-	Refill
11:39	10	34	10	1.00	Test
		Water Drained	d within 10 Minut	es - Start Percolation Test	
11:40	-	28	-	-	Start
11:50	10	32 1/2	4 1/2	2.22	Test
11:50	-	28	-	-	Refill
12:00	10	31	3	3.33	Test
12:00	-	28	-	-	Refill
12:10	10	31	3	3.33	Test
12:10	-	28	-	-	Refill
12:15	5	29 1/4	1 1/4	4.00	Test
12:15	-	28	-	-	Refill
12:20	5	29 1/4	1 1/4	4.00	Test
12:20	-	28	-	-	Refill
12:25	5	29 1/4	1 1/4	4.00	Test
12:25	-	28	-	-	Refill
12:30	5	29 1/4	1 1/4	4.00	Test
12:30	-	28	-	-	Refill
12:35	5	29 1/4	1 1/4	4.00	Test
12:35	-	28	-	-	Refill
12:40	5	29 1/2	1 1/4	4.00	Test/Finish

SOIL DATA

0.0-1.0' FILL (SM) Dark Brown Silty Sand with Gravel; Slightly Moist 1.0-3.0' (SP-SC) Light Brown Poorly Graded Sand with Clay and Gravel; Slightly Moist

TEST NUMBER P-4

PROJECT NAME	Halloran Summit Travel Stati	on DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road		
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	N/A - Fast Perc	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3	-	

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)	\$ F	(2)	(1) / (2)	
1:40	-	24	-	-	Start
1:48	8	34	10	0.80	Test
1:48	-	24	-	-	Refill
1:57	9	34	10	0.90	Test
		Water Drained	d within 10 Minute	es - Start Percolation Test	
1:57	-	28	-	-	Start
2:02	5	31	3	1.67	Test
2:02	-	28	-	-	Refill
2:07	5	30 3/4	2 3/4	1.82	Test
2:07	-	28	-	-	Refill
2:12	5	30 1/2	2 1/2	2.00	Test
2:12	-	28	-	-	Refill
2:17	5	30 1/4	2 1/4	2.22	Test
2:17	-	28	-	-	Refill
2:22	5	30	2	2.50	Test
2:22	-	28	-	-	Refill
2:27	5	29 3/4	1 3/4	2.86	Test
2:27	-	28	-	-	Refill
2:32	5	29 1/2	1 1/2	3.33	Test
2:32	-	28	-	-	Refill
2:37	5	29 1/2	1 1/2	3.33	Test
2:37	-	28	-	-	Refill
2:42	5	29 1/4	1 1/4	4.00	Test
2:42	-	28	-	-	Refill
2:47	5	29 1/4	1 1/4	4.00	Test
2:47	-	28	-	-	Refill
2:52	5	29 1/4	1 1/4	4.00	Test
2:52	-	28	-	-	Refill
2:57	5	29 1/4	1 1/4	4.00	Test/Finish

SOIL DATA

0.0-1.0' FILL (SM) Dark Brown Silty Sand with Gravel; Slightly Moist 1.0-3.0' (SP-SC) Light Brown Poorly Graded Sand with Clay and Gravel; Slightly Moist

TEST NUMBER P-5

PROJECT NAME	Halloran Summit Travel Sta	ation DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road		
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	22	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3	_	

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)		(2)	(1) / (2)	
11:34	-	24	-	-	Start
11:44	10	30	6	1.67	Test
11:44	-	24	-	-	Refill
11:54	10	32	4	2.50	Test
	Did Not Drain C	out in 10 Minute	s - Presoak Proc	eedure - Maintained 10" Head	or at least 4 Hours
9:10	-	28	-	-	Start
9:40	30	28 1/4	1/4	120.00	Test
10:40	60	28 5/8	3/8	160.00	Test
11:40	60	29	3/8	160.00	Test
12:40	60	29 3/8	3/8	160.00	Test/Finish

SOIL DATA

0.0-3.0' FILL (SC) Brown Clayey Sand with Gravel; Slightly Moist

TEST NUMBER P-6

PROJECT NAME	Halloran Summit Travel Statio	on DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road		
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	N/A - Fast Perc	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3	_	

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)		(2)	(1) / (2)	
11:19	-	24	-	-	Start
11:29	10	34	10	1.00	Test
11:29	-	24	-	-	Refill
11:39	10	34	10	1.00	Test
		Water Drained	d within 10 Minut	es - Start Percolation Test	
11:39	-	28	-	-	Start
11:49	10	32	6	1.67	Test
11:49	-	28	-	-	Refill
11:59	10	32	6	1.67	Test
11:59	-	28	-	-	Refill
12:04	5	30 1/2	2 1/2	2.00	Test
12:04	-	28	-	-	Refill
12:09	5	30 1/2	2 1/2	2.00	Test
12:09	-	28	-	-	Refill
12:14	5	30 3/8	2 3/8	2.11	Test
12:14	-	28	-	-	Refill
12:19	5	30 3/8	2 3/8	2.11	Test
12:19	-	28	-	-	Refill
12:24	5	30 3/8	2 3/8	2.11	Test
12:24	-	28	-	-	Refill
12:29	5	30 3/8	2 3/8	2.11	Test
12:29	-	28	-	-	Refill
12:34	5	30 3/8	2 3/8	2.11	Test
12:34	-	28	-	-	Refill
12:39	5	30 3/8	2 3/8	2.11	Test/Finish

SOIL DATA

0.0-1.0' FILL (SM) Dark Brown Silty Sand with Gravel; Slightly Moist 1.0-3.0' (SP-SC) Light Brown Poorly Graded Sand with Clay and Gravel; Slightly Moist

TEST NUMBER P-8

PROJECT NAME	Halloran Summit Travel Statior	DATE	2/2/2024
PROJECT LOCATION	Halloran Summit Road		
TEST LOCATION	See Site Plan	REVIEWED BY	
TEST PERFORMED BY	C. O'Neill	REGISTRATION NO.	

TEST DATA

HOLE DIAMETER (inches)	8	DEPTH OF WATER IN HOLE	
TIME OF PRESOAKING (hours)	21	AT START OF TEST (inches)	10
TEST DEPTH (feet)	3	-	

	TIME	DEPTH TO	DEPTH	PERCOLATION	
TIME OF	DIFFERENCE	WATER	DIFFERENCE	RATE	COMMENTS
READING	(minutes)	(inches)	(inches)	(minutes/inche)	
	(1)		(2)	(1) / (2)	
1:36	-	24	-	-	Start
1:46	10	30	6	1.67	Test
1:46	-	24	-	-	Refill
1:56	10	28	4	2.50	Test
Water	Did Not Drain C	out in 10 Minute	s - Presoak Proc	eedure - Maintained 10" Head	for at least 4 Hours
11:20	-	28	-	-	Start
11:50	30	31	3	10.00	Test
11:50	-	28	-	-	Refill
12:20	30	30 1/2	2 1/2	12.00	Test
12:20	-	28	-	-	Refill
12:50	30	29 3/4	1 3/4	17.14	Test
12:50	-	28	-	-	Refill
1:20	30	29 1/2	1 1/2	20.00	Test
1:20	-	28	-	-	Refill
1:50	30	29	1	30.00	Test
1:50	-	28	-	-	Refill
2:20	30	29	1	30.00	Test/Finish

SOIL DATA

0.0-3.0' (SC) Light Brown Clayey Sand with Gravel; Slightly Moist